

STATEMENT OF SPECIAL INSPECTIONS

PROJECT: New Freestanding Mavis Tires & Brakes
 LOCATION: 4391 Harley Bridge Rd. Macon, GA 31216 (Bibb County)
 PERMIT APPLICANT: Capital Growth Buchalter, LLC - Kirk Farrelly (Owner/Developer)
 APPLICANT'S ADDRESS: 361 Summit Blvd Suite 110 Birmingham, AL 35243
 ARCHITECT OF RECORD: Joseph L. Oliveri, AIA, Oliveri Architects
 STRUCTURAL ENGINEER OF RECORD: H. Eugene Hunter, P.E.
 MECHANICAL ENGINEER OF RECORD: Joseph H. Griner III, P.E., Griner Engineering, Inc.
 ELECTRICAL ENGINEER OF RECORD: Joseph H. Griner III, P.E., Griner Engineering, Inc.
 REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE: H. Eugene Hunter, P.E.

This Statement of Special Inspections is submitted in accordance with Section 1704.3 of the 2018 International Building Code. It includes a *Schedule of Special Inspection Services* applicable to the above-referenced Project as well as the identity of the individuals, agencies, or firms intended to be retained for conducting these inspections. If applicable, it includes *Special Inspections for Seismic Resistance* and/or *Special Inspections for Wind Resistance*.

Are *Special Inspections for Seismic Resistance* included in the *Statement of Special Inspections*? ☐ Yes ☒ No
 Are *Special Inspections for Wind Resistance* included in the *Statement of Special Inspections*? ☐ Yes ☒ No

The Special Inspector(s) shall keep records of all inspections and shall furnish interim inspection reports to the Building Official and to the Registered Design Professional in Responsible Charge at a frequency agreed upon by the Design Professional and the Building Official prior to the start of work. Discrepancies shall be brought to the immediate attention of the Contractor for correction. If the discrepancies are not corrected, the discrepancies shall be brought to the attention of the Building Official and the Registered Design Professional in Responsible Charge prior to completion of that phase of work. A *Final Report of Special Inspections* documenting required special inspections and corrections of any discrepancies noted in the inspections shall be submitted to the Building Official and the Registered Design Professional in Responsible Charge at the conclusion of the project, signed and sealed by the Special Inspection Engineer.

Frequency of interim report submittals to the Registered Design Professional in Responsible Charge:
 Weekly ☐ Bi-Weekly ☒ Monthly ☐ Other, specify: _____

The Special Inspection program does not relieve the Contractor of the responsibility to comply with the Contract Documents. Jobsite safety and means and methods of construction are solely the responsibility of the Contractor.

Statement of Special Inspections Prepared by:

H. Eugene Hunter, P.E.

Type or print name

11-11-2022

Signature

Date

Building Official's Acceptance:

Signature

Date

Permit Number:

Frequency of interim report submittals to the Building Official:

Monthly ☐ Bi-Monthly ☐ Upon Completion ☒ Other, specify: _____

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SCHEDULE OF SPECIAL INSPECTIONS SERVICES					
PROJECT	MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT		
			Y/N	EXTENT	AGENT* DATE COMPLETED
1705.1.1 Special Cases (work unusual in nature, including but not limited to alternative materials and systems, unusual design applications, materials and systems with special manufacturer's requirements - add additional rows as needed.)	Submittal review, shop (3) and/or field inspection		N		
	Field inspection	Periodic or as required by the research report issued by an approved source			
1705.2.1 Structural Steel Construction	Submittal Review	Each submittal	Y		
	Shop (3) and field inspection	Periodic	Y		
1705.2.2 Cold-Formed Steel Deck	Shop (3) and field inspection	Periodic	Y		
	Shop (3) and field inspection	Observe or Perform as noted (4)	Y		
1705.2.3 Open-Web Steel Joists and Joist Girders	Shop (3) and field inspection	Observe (4)	Y		
	Shop (3) and field inspection	Observe (4)	Y		
1705.2.4 Cold-Formed Steel Trusses Spanning 60 feet or Greater	Shop (3) and field inspection	Observe or Perform as noted (4)	Y		
	Shop (3) and field inspection	Observe or Perform as noted (4)	Y		
1705.3 Concrete Construction	Shop (3) or field ultrasonic testing - 100%	Periodic	N		

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SCHEDULE OF SPECIAL INSPECTIONS SERVICES					
PROJECT	MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT		
			Y/N	EXTENT	AGENT* DATE COMPLETED
2) Complete penetration groove welds 5/16" or greater in risk category II	Shop (3) or field ultrasonic testing - 10% of welds minimum	Periodic	Y		
	Shop (3) or field radiographic or ultrasonic testing	Periodic	N		
3) Welded joints subject to fatigue when required by AISC 360, Appendix 3, Table A-3.1	Verify reports	Each submittal (5)	Y		
	Shop (3) and field inspection	Observe or Perform as noted (4)	Y		
4) Fabricator's NDT reports when fabricator performs NDT	Shop (3) and field inspection	Observe or Perform as noted (4)	Y		
	Shop (3) and field inspection	Observe (4)	Y		
5) Visual inspection of exposed out surfaces of galvanized structural steel main members and exposed corners of the rectangular HSS for cracks subsequent to galvanizing	Shop (3) or field inspection	Periodic	Y		
	Field inspection	Periodic	Y		
6) Embedments (Verify diameter, grade, type, length, embedment. See 1705.3 for anchors)	Field inspection	Periodic	Y		
	Field inspection	Periodic	Y		

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SCHEDULE OF SPECIAL INSPECTIONS SERVICES					
PROJECT	MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT		
			Y/N	EXTENT	AGENT* DATE COMPLETED
b. Inspection tasks During Welding (Observe the QA tasks listed in SDI QA/QC, Appendix 1 Table 1.4)			Y	Observe (4)	
			Y	Perform (4)	
c. Inspection tasks After Welding (Perform the QA tasks listed in SDI QA/QC, Appendix 1 Table 1.5)	Shop (3) and field inspection		Y		
			Y	Observe (4)	
5. Cold-formed steel deck mechanical fastening	Shop (3) and field inspection		Y		
			Y	Observe (4)	
a. Inspection tasks Prior to Mechanical Fastening (Observe the QA tasks listed in SDI QA/QC, Appendix 1 Table 1.6)			Y	Observe (4)	
			Y	Observe (4)	
b. Inspection tasks During Mechanical Fastening (Observe the QA tasks listed in SDI QA/QC, Appendix 1 Table 1.7)			Y	Perform (4)	
			Y	Perform (4)	
c. Inspection tasks After Mechanical Fastening (Perform the QA tasks listed in SDI QA/QC, Appendix 1 Table 1.8)			Y	Perform (4)	
			Y	Perform (4)	

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SCHEDULE OF SPECIAL INSPECTIONS SERVICES					
PROJECT	MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT		
			Y/N	EXTENT	AGENT* DATE COMPLETED
6. Prior to placement, fresh concrete sampling, slump and air content tests and determine temperature of concrete and perform any other tests as specified in construction documents	Shop (3) and field inspection		Y	Continuous	
			Y	Continuous	
7. Inspection of concrete and shotcrete placement for proper application techniques	Shop (3) and field inspection		Y	Periodic	
			N	Continuous	
8. Verify maintenance of specified curing temperature and techniques	Shop (3) and field inspection		Y	Periodic	
			N	Continuous	
9. Inspection of prestressed concrete	Shop (3) and field inspection		N	Continuous	
			N	Continuous	
10. Inspect erection of precast concrete members	Shop (3) and field inspection		N	Periodic	
			N	Continuous	
11. Verification of in-situ concrete strength, prior to stressing of tendons in post tensioned concrete and prior to removal of shores and forms from beams and structural slabs	Review field testing and laboratory reports		N	Periodic	
			N	Periodic	
12. Inspection of formwork for shape, lines, location and dimensions	Field inspection		Y	Periodic	
			Y	Periodic	
13. Concrete strength testing and verification of compliance with construction documents	Field testing and review of laboratory reports		Y	Periodic	
			Y	Periodic	

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SCHEDULE OF SPECIAL INSPECTIONS SERVICES					
PROJECT	MATERIAL / ACTIVITY	SERVICE	APPLICABLE TO THIS PROJECT		
			Y/N	EXTENT	AGENT* DATE COMPLETED
d. Prestressing technique	Field inspection		N	Periodic	
	Field inspection		N	Level 2 - Continuous ^(b) Level 2 - Periodic ^(c)	
e. Properties of thin-bed mortar for AAC masonry	Field inspection		N	Level 3 - Continuous	
	Field inspection		N	Level 2 - Periodic Level 3 - Continuous	
1. Sample panel construction	Field inspection		N	Level 2 - Periodic Level 3 - Continuous	
			N	Level 3 - Continuous	
2. Prior to grouting, verify that the following are in compliance:	Field inspection		Y	Level 2 - Periodic Level 3 - Continuous	
	Field inspection		N	Periodic	
3. Verify compliance of the following during construction:	Field inspection		Y	Periodic	
	Field inspection		Y	Periodic	
4. Observe preparation of grout specimens, mortar specimens, and/or prisms	Field inspection		Y	Periodic	
			N	Level 2 - Periodic Level 3 - Continuous	

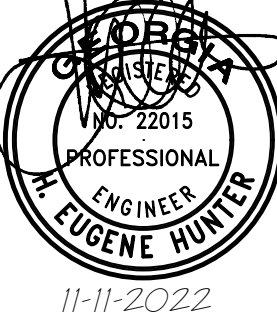
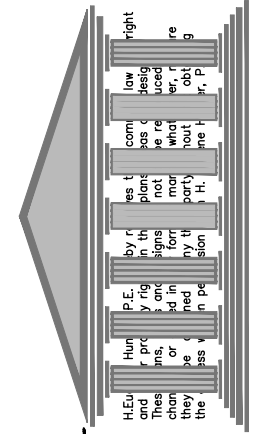
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NOTE: SEE 8.5"x11" SPECIAL INSPECTION PACKAGE FOR ALL SHEETS

REVISIONS BY

OLIVERI ARCHITECTS



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New Freestanding
 MAVIS TIRES & BRAKES
 4391 Harley Bridge Rd.
 Macon, Georgia 31216
 Bibb County

Date: 11. 11. 22

Scale: AS NOTED

Project Mgr: MD

Drawn: CAD

Job: 4601

Sheet

S0.1

BASED ON 8-BAY SINGLE SIDED PROTO, DATED APRIL 01 2022

86 of 83069 of 82027 of 810820 of 81088 of 810

BASED ON 8-BAY SINGLE SIDED PROTO. DATED APRIL 01 2022

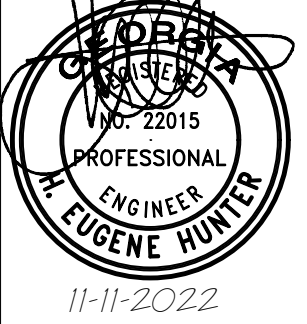
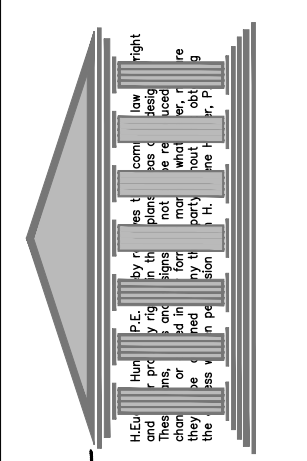
REVISIONS	BY



AA-0002021

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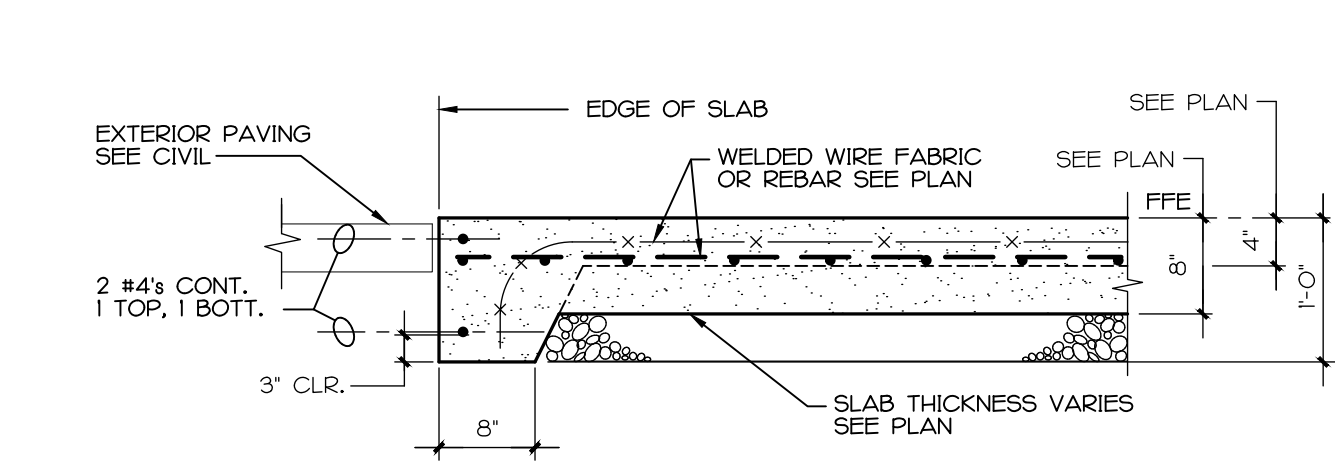
New Freestanding

MAVIS TIRES & BRAKES

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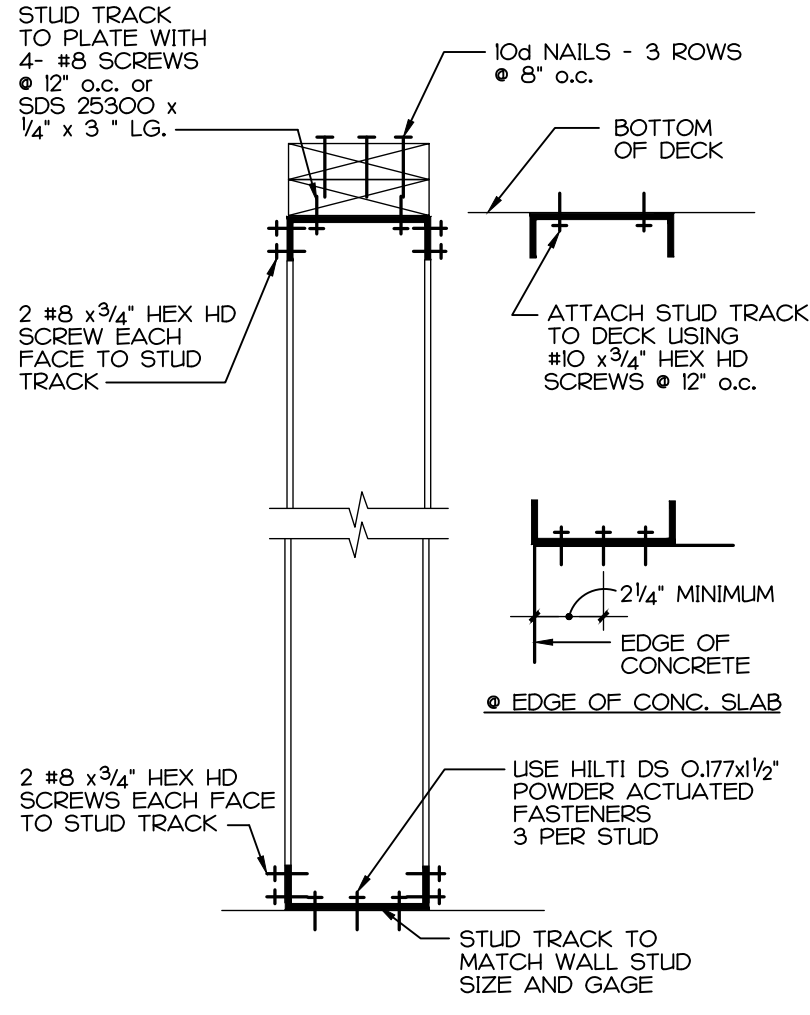
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Scale:	AS NOTED
Project Mgr:	MD
Drawn:	CAD
Job:	4601
Sheet	S1



4

SECTION OF EXT. SLAB

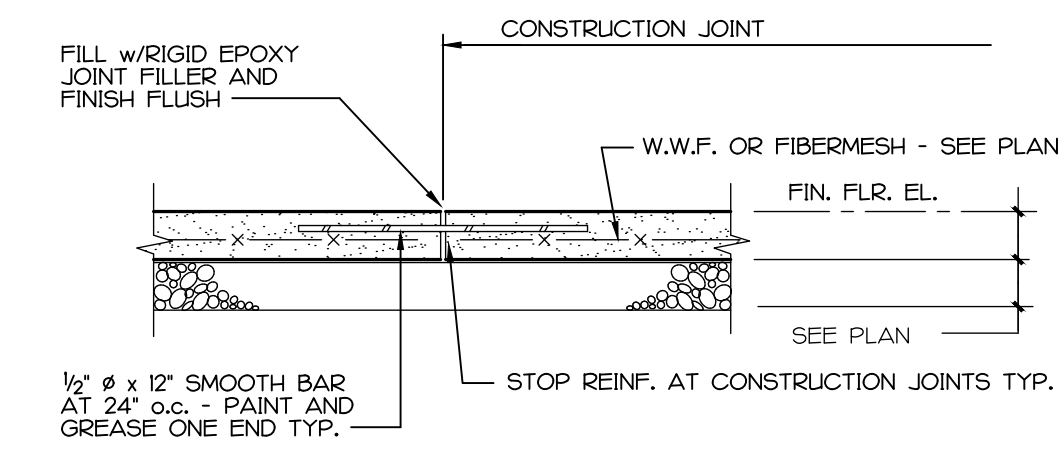
3/4" = 1'-0"



8

SECTION OF STUD BASE

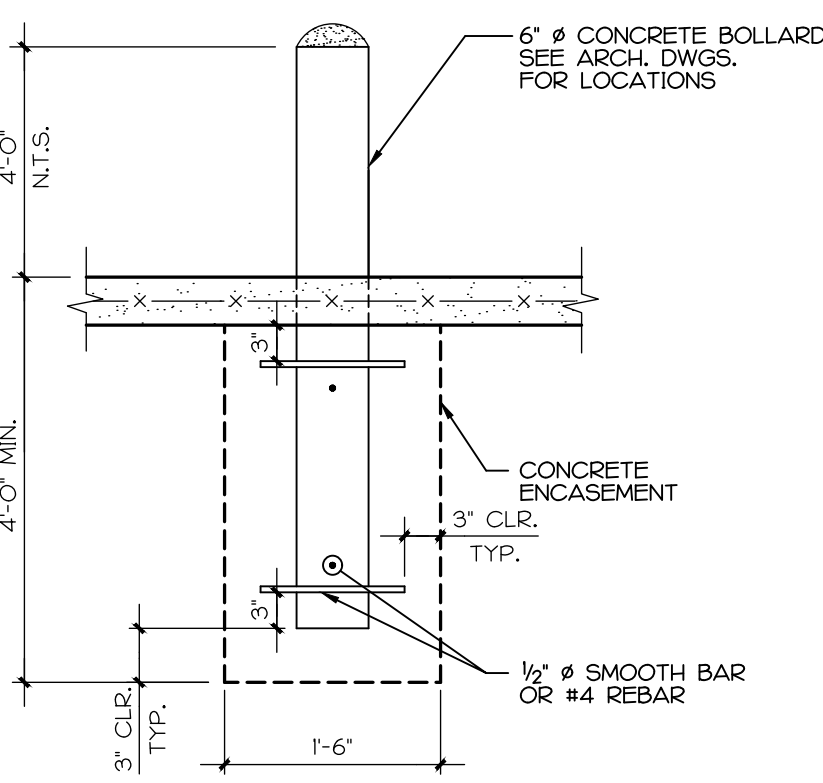
3/4" = 1'-0"



3

SECTION AT CONSTRUCTION JT

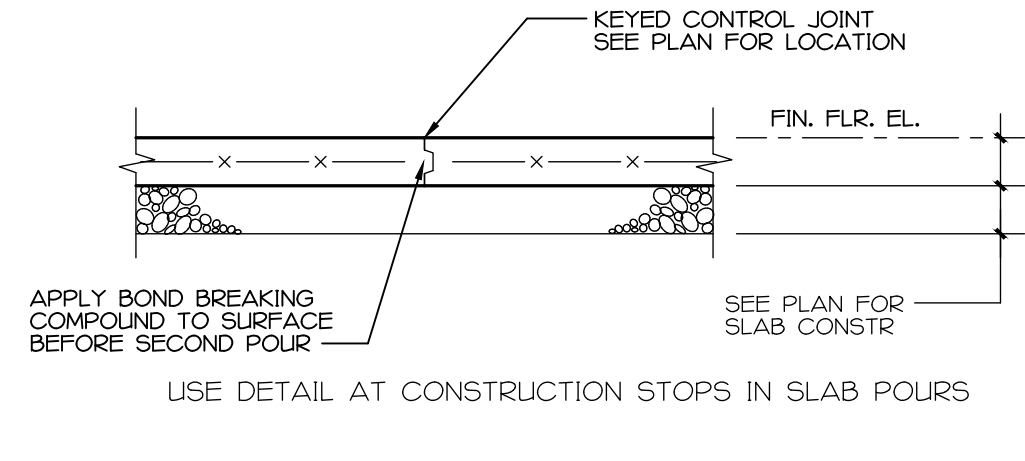
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7

TYPICAL BOLLARD DETAIL

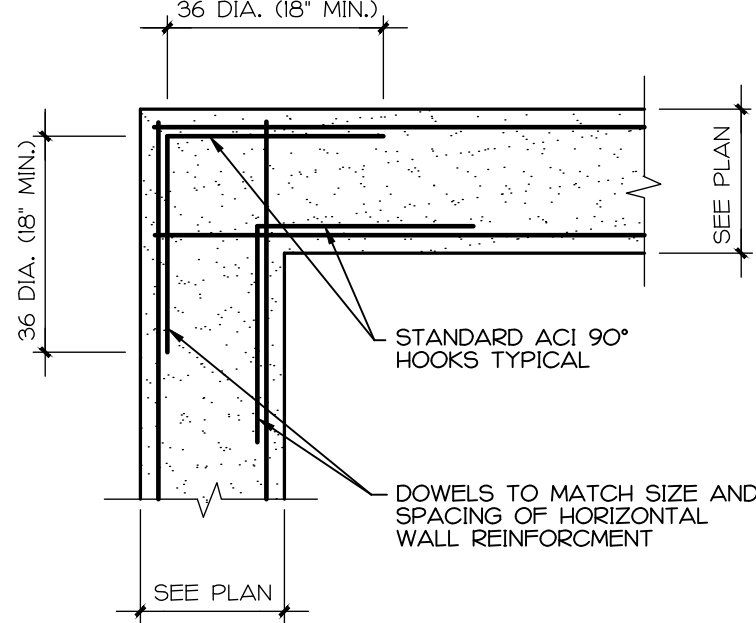
3/4" = 1'-0"



2

SECTION AT KEYED JOINT

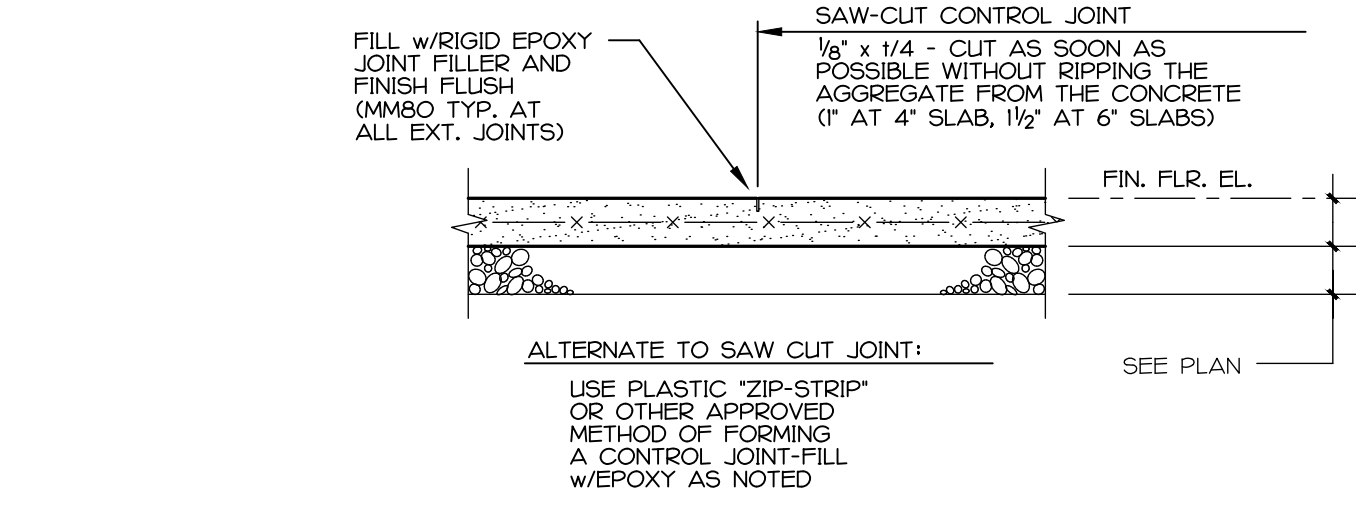
NO SCALE



6

SECTION AT TYPICAL WALL/ FTG CORNER BAR DETAILS

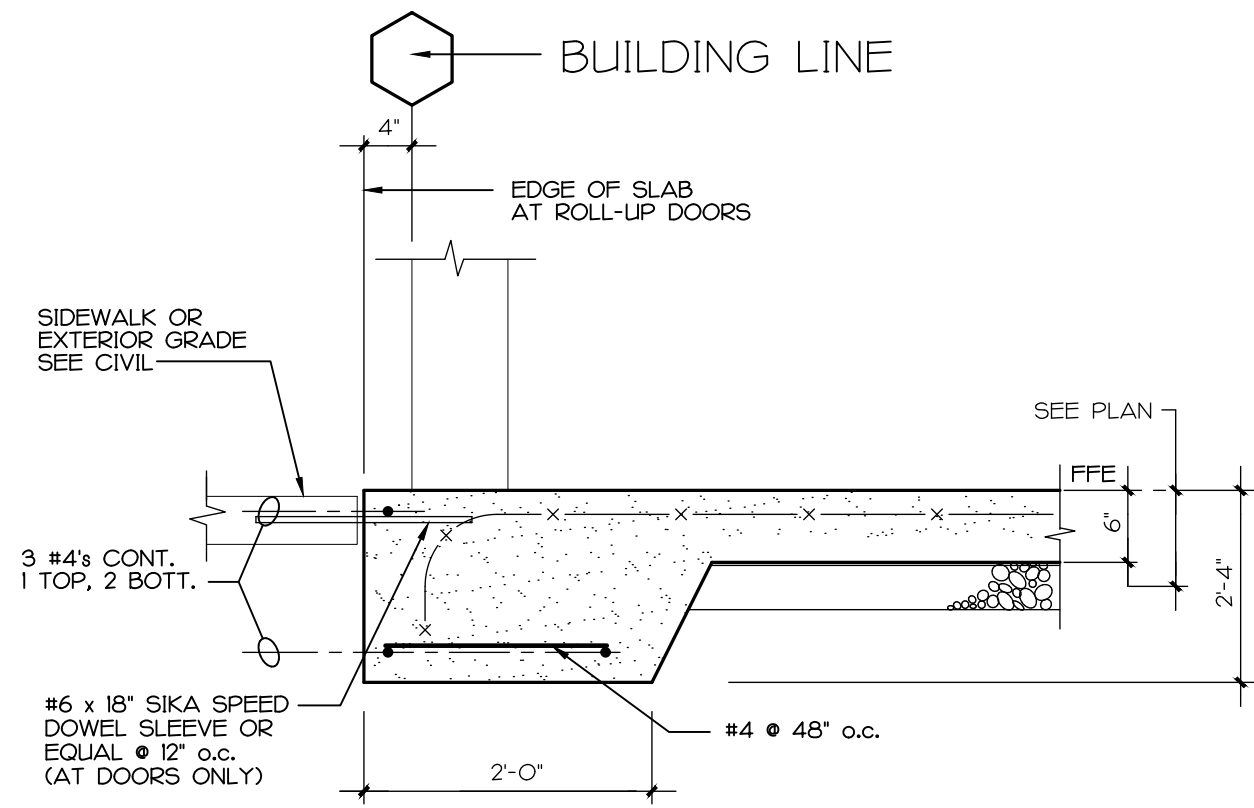
3/4" = 1'-0"



1

SECTION AT SAW-CUT JOINT

NO SCALE



5

SECTION

3/4" = 1'-0"

I. GENERAL		2. EARTHWORK AND FOUNDATIONS		5. METALS		5. METALS (CONT.)	
11. PROJECT LOCATION		21. EARTHWORK AND FOUNDATION DESIGN IS BASED UPON THE GEOTECHNICAL REPORT ENTITLED G22CSP04 BY CONTOUR ENGINEERING, LLC DATED APRIL, 20, 2022.		51. STRUCTURAL STEEL - REFER TO SPECIFICATION SECTION 052100 AND THE REQUIREMENTS BELOW.		52. STEEL JOISTS - REFER TO SPECIFICATION 052100	
A. ADDRESS.....4391 HARTLEY BRIDGE RD MACON, GA 31216		22. THE CONTRACTOR SHALL PROVIDE AND OPERATE DEWATERING EQUIPMENT AND BE RESPONSIBLE FOR MAINTAINING EXCAVATIONS AND WORK AREAS IN A DRY CONDITION. WATER SHALL BE DISCHARGED TO A LOCATION SUITABLE TO THE SER AND IN ACCORDANCE WITH APPLICABLE LOCAL, STATE AND FEDERAL LAWS AND REGULATIONS.		A. STANDARDS		53. STEEL DECKING - REFER TO SPECIFICATION 053100	
B. MUNICIPALITY....CITY OF MACON - BIBBS COUNTY		23. ARRANGE FOR OWNER'S INDEPENDENT TESTING AGENCY TO MONITOR CUT AND FILL OPERATIONS AND PERFORM FIELD DENSITY AND MOISTURE CONTENT TESTS TO VERIFY COMPACTION AND APPROVE FOOTING SUBGRADES PRIOR TO PLACING CONCRETE.		1. AMERICAN INSTITUTE OF STEEL CONSTRUCTION, INC. (AISC)-SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, (AISC 360-16)		54. COLD-FORMED METAL FRAMING AND TRUSSES - REFER TO SPECIFICATION SECTION 054000 AND THE REQUIREMENTS BELOW.	
12. BUILDING CODES AND STANDARDS		24. BACKFILL		A. MATERIALS		A. STANDARDS	
A. GEORGIA STATE MINIMUM BUILDING CODE		A. ALL BACKFILL AND TRENCHING OPERATIONS SHALL COMPLY WITH ALL CURRENT AND APPLICABLE LOCAL, STATE AND FEDERAL SAFETY CODES, INCLUDING THE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION.		1. WIDE FLANGE AND WT SHAPES.....ASTM A992, GRADE 50		1. AMERICAN IRON AND STEEL INSTITUTE (AISI) NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS (AISI S100-16)	
B. INTERNATIONAL BUILDING CODE (IBC), 2018		B. THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING ADEQUATE SHORING OF THE NEW AND EXISTING CONSTRUCTION DURING CONSTRUCTION OPERATIONS IN ORDER TO PREVENT ANY DAMAGE DUE TO BACKFILLING AND TRENCHING.		2. ANGLES, CHANNELS AND PLATES.....ASTM A36		2. (AISI) NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING -GENERAL PROVISIONS (AISI S202-15)	
C. AMERICAN SOCIETY OF CIVIL ENGINEERS (ASCE) - MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES, 2018 EDITION (ASCE 7-16)		25. STRUCTURAL FILL		3. HOLLOW STRUCTURAL SHAPES.....ASTM A1085		3. (AISI) NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING -FLOOR AND ROOF SYSTEM DESIGN (AISI S240-15)	
13. OCCUPANCY		A. STANDARDS		4. ANCHOR RODS.....ASTM F1554, 36 KSI		4. (AISI) NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING -WALL STUD DESIGN (AISI S240-15)	
A. CLASSIFICATION.....RE: ARCH		6. STATE DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR HIGHWAY CONSTRUCTION		5. BOLTS:		5. (AISI) NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING -HEADER DESIGN (AISI S240-15)	
B. CATEGORY.....II		B. MATERIALS		a. HIGH STRENGTH.....ASTM A325, TYPE I (PROVIDE (2) 3/4" DIA MINIMUM PER CONNECTION)		6. (AISI) NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING -LATERAL DESIGN (AISI S240-15)	
14. DESIGN LOADS		1. GENERAL AGGREGATE.....STATE DOT SPECIFICATIONS		6. NUTS.....ASTM A563, GRADE C		7. (AISI) NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING -TRUSS DESIGN, WITH SUPPLEMENT 2, DATED 2008 (AISI S24-07)	
A. DEAD LOADS		2. CONTROLLED LOW STRENGTH MATERIAL.....STATE DOT SPECIFICATIONS		7. HARDENED WASHERS.....ASTM F436, TYPE I		8. (AISI) NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING -SEISMIC DESIGN, WITH SUPPLEMENT 1, DATED 2016 (AISI 2400-15)	
I. ROOF.....20 PSF		C. EXECUTION		8. PLATE WASHERS.....ASTM A36 (360-16); TABLE I4-2		9. AWS STRUCTURAL WELDING CODE - SHEET STEEL (AWS D1.3-98)	
B. LIVE LOADS		1. AFTER ACHIEVING FINISHED SUBGRADE IN CUT AREAS AND PRIOR TO PLACING STRUCTURAL FILL IN AREAS BELOW FINISHED SUBGRADE, THE EXPOSED SUBGRADE SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER OF RECORD TO CONFIRM THAT ALL UNSUITABLE OR UNSTABLE MATERIALS HAVE BEEN REMOVED.		9. WELD ELECTRODES.....AWS CLASS E70XX (LOW HYDROGEN)		B. MATERIALS	
1. ROOF (MINIMUM).....20 PSF		2. UNLESS NOTED OTHERWISE IN THE GEOTECHNICAL REPORT, ALL STRUCTURAL FILL SHALL BE PLACED IN HORIZONTAL LIFTS NOT EXCEEDING EIGHT (8) INCHES IN LOOSE THICKNESS AND WITHIN THREE (3) PERCENT OF OPTIMUM MOISTURE FOR COMPACTION. THE STRUCTURAL FILL SHALL BE COMPACTED TO 98 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY STANDARD PROCTOR (ASTM D698).		10. GROUT.....ASTM C107		1. ZINC COATED STEEL SHEET.....ASTM A1003	
2. SLAB ON GRADE.....100 PSF		3. MAINTAIN SUBGRADE AND STRUCTURAL FILL MOISTURE CONTENT UNTIL FOUNDATIONS ARE PLACED.		(NON-METALLIC AND SHRINKAGE RESISTANT WITH A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 6,000 PSI)		2. LOAD-BEARING COLD-FORMED MEMBERS.....ASTM C955-07	
C. SNOW LOADS		2.6 GRANULAR BASE MATERIAL		C. EXECUTION		a. STEEL STUDS:	
1. GROUND SNOW LOAD (Pg).....5 PSF		A. FOUR (4) INCHES OF GRANULAR BASE MATERIAL SHALL BE PLACED UNDER ALL CONCRETE SLABS-ON-GRADE.		1. DESIGN AND DETAIL STRUCTURAL STEEL ELEMENTS IN ACCORDANCE WITH ALL APPLICABLE STANDARDS. STRUCTURAL STEEL ERECTION SHALL COMPLY WITH ALL FEDERAL, STATE AND LOCAL SAFETY REGULATIONS.		1) MINIMUM UNCOATED STEEL THICKNESS.....0.0428 INCH	
2. SNOW LOAD IMPORTANCE FACTOR (Is).....1.0		B. MATERIAL		2. PRIOR TO FABRICATION, STEEL FABRICATOR SHALL PREPARE AND SUBMIT AN ELECTRONIC (PDF) COPY OF SHOP DRAWINGS (ERECTION AND DETAILS) TO THE SER FOR REVIEW AND APPROVAL. SHOP DRAWINGS SHALL BE SEALED BY A PROFESSIONAL ENGINEER THAT IS REGISTERED IN THE STATE WHERE THE PROJECT IS LOCATED. FABRICATOR IS RESPONSIBLE FOR ALL ERRORS ON THE SHOP DRAWINGS, ERRORS DURING FABRICATION AND THE CORRECT FITTING OF STRUCTURAL STEEL MEMBERS.		2) MINIMUM FLANGE WIDTH.....5/8 INCHES	
3. TEMPERATURE FACTOR (Ct).....1.0		1. INTERIOR SLABS-ON-GRADE.....STATE DOT SPECIFICATIONS		3. PROVIDE WASHERS FOR ALL CONNECTIONS THAT INCLUDE STANDARD, OVERSIZE AND SHORT-SLOTTED HOLES. 5/16" THICK MINIMUM PLATE STEEL (A36) WASHERS SHALL BE PROVIDED FOR ALL LONG-SLOTTED HOLES AND HOLES FOR ANCHOR RODS UNLESS NOTED. PLATE WASHERS SHALL COMPLETELY COVER ALL LONG-SLOTTED HOLES.		b. STEEL TRACK:	
4. EXPOSURE FACTOR (Ce).....1.0		2. EXTERIOR SLABS-ON-GRADE.....AASHTO NO. 57 AGGREGATE		4. PROVIDE MINIMUM SIZE FILLET WELDS AND MINIMUM EFFECTIVE THROAT THICKNESSES OF PARTIAL PENETRATION GROOVE WELDS AS SPECIFIED BY SPECIFICATION SECTION "J" OF THE AISC MANUAL. DEVELOP THE FULL TENSILE STRENGTH OF THE MEMBER(S) JOINED ON ALL SHOP AND FIELD WELDS UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS. ALL WELD LENGTHS ARE CONTINUOUS FOR THE FULL LENGTH OF THE MEMBER(S) UNLESS NOTED OTHERWISE ON THE DESIGN DRAWINGS.		1) MINIMUM UNCOATED STEEL THICKNESS.....MATCH STUD	
5. FLAT ROOF SNOW LOAD (Pt).....3.2 PSF		3. CONCRETE		5. FABRICATOR SHALL BE FREE OF MILL SCALE, RUST, OIL, GREASE OR ANY OTHER FOREIGN MATTER. IN ADDITION, ALL STEEL SURFACES TO BE PAINTED SHALL BE PREPARED IN ACCORDANCE TO SSPC-SP2, "HAND TOOL CLEANING"		2) MINIMUM FLANGE WIDTH.....1/4 INCHES	
D. WIND LOADS		4. MASONRY		a. PRIOR TO AND DURING FABRICATION PROCEDURES, ALL STRUCTURAL STEEL SHALL BE FREE OF MILL SCALE, RUST, OIL, GREASE OR ANY OTHER FOREIGN MATTER. IN ADDITION, ALL STEEL SURFACES TO BE PAINTED SHALL BE PREPARED IN ACCORDANCE TO SSPC-SP2, "HAND TOOL CLEANING"		c. STEEL DEFLECTION TRACK:	
1. ULTIMATE WIND SPEED (Vw1), 3 SECOND GUST.....107 MPH		41. REFER TO SPECIFICATION SECTION 042000.		b. FABRICATE AND ASSEMBLE STRUCTURAL STEEL MEMBERS AND ASSEMBLIES. THE SHOP DRAWINGS SHALL BE SEALED BY THE FABRICATOR. WELDING PROCEDURES SHALL BE PERFORMED AND SEQUENCED SO AS TO MINIMIZE WELD SHRINKAGE STRESSES AND DISTORTION OF THE MEMBERS.		1) MINIMUM UNCOATED STEEL THICKNESS.....0.0538 INCH	
2. WIND IMPORTANCE FACTOR.....1.0				c. PROVIDE SIMPLE SHEAR CONNECTIONS FOR STRUCTURAL STEEL CONNECTIONS NOT SPECIFIED OTHERWISE, UTILIZING HIGH STRENGTH BEARING BOLTS IN SINGLE OR DOUBLE SHEAR. PROVIDE DOUBLE ANGLE OR SINGLE PLATE SHEAR TAB BOLTED CONNECTIONS, UNLESS A LARGER REACTION IS SHOWN ON THE DRAWINGS, CONNECTIONS SHALL DEVELOP THE FULL STRENGTH OF THE MEMBER.		2) MINIMUM FLANGE WIDTH.....2 INCHES	
3. WIND EXPOSURE.....C				d. FINISH STEEL COLUMN ENDS TO FIT FLUSH WITH BASE AND CAP PLATES. FINISH BEAM ENDS TO FIT FLUSH WITH END PLATES. FIELD ASSEMBLY OF THESE PLATE ELEMENTS IS NOT PERMITTED.			
4. INTERNAL PRESSURE COEFFICIENT.....-10.18				e. BRACE CONNECTIONS SHALL BE PROVIDED PER THE DETAILS SHOWN ON THE DESIGN DRAWINGS, WHERE FORCES ARE NOT INDICATED ON THE DESIGN DRAWINGS, DESIGN CONNECTIONS OF BRACING MEMBERS THAT DEVELOP THE FULL TENSILE STRENGTH OF THE MEMBERS, ALL GUSSET PLATE, ANGLE AND WELD SIZES SHALL BE DESIGNED TO RESIST ALL TENSILE, SHEAR AND MOMENT FORCES CAUSED BY CONCENTRIC AND ECCENTRIC LOADING CONDITIONS, ALL BOLTED CONNECTIONS FOR BRACING MEMBERS SHALL BE FULLY TENSIONED, UTILIZING HIGH STRENGTH CLASS A "SLIP-CRITICAL" BOLTS.			
5. COMPONENT & CLADDING DESIGN WIND PRESSURE				f. WITH THE EXCEPTION OF CONTACT SURFACES FOR "SLIP- CRITICAL" CONNECTIONS AND STRUCTURAL STEEL TO BE EMBEDDED IN CONCRETE, ALL STRUCTURAL STEEL SHALL RECEIVE ONE COAT OF RUST-INHIBITIVE PRIMER AS SELECTED AND APPROVED BY THE OWNER WITH A MINIMUM 15 MILS DRY FILM THICKNESS. FURNISH SHOP PRIMER TO THE ERECTOR IN SUFFICIENT QUANTITY FOR FIELD-TOUCH OF ALL FIELD WELDS AND ABRASIONS. OMIT PRIMER ON STEEL SURFACES WHERE SPRAY-ON FIRE-PROOFING IS REQUIRED.			
a. ROOF PRESSURE (10 SF).....-66.4 PSF, +10.7 PSF				g. USE PROCEDURES, INCLUDING TEMPORARY BRACES OR GUYS, AS REQUIRED AT ALL TIMES TO MAINTAIN SAFETY AND STABILITY OF THE STRUCTURE. TEMPORARY BRACING PROCEDURES IS THE RESPONSIBILITY OF THE CONTRACTOR AND SHALL REMAIN IN PLACE PRIOR TO THE COMPLETE INSTALLATION OF ALL PERMANENT BRACING ELEMENTS AND SYSTEMS.			
b. WALL PRESSURE (10 SF).....-32.1 PSF, +24.1 PSF				h. FIELD CORRECTIONS AND GAS CUTTING OF FABRICATED STRUCTURAL STEEL MEMBERS IS NOT PERMITTED. SPLICING OF STRUCTURAL STEEL MEMBERS CAN NOT BE PERFORMED WITHOUT PRIOR APPROVAL OF THE SER.			
E. EARTHQUAKE DESIGN DATA				i. INSTALL GROUT UNDER COLUMN BASE PLATES AND BEARING PLATES IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS. FILL ENTIRE GROUT SPACE SO AS TO PROVIDE FULL PLATE BEARING.			
1. RISK CATEGORY.....II				j. ALL STRUCTURAL STEEL SURFACES TO BE FIELD WELDED SHALL BE PREPARED AND CLEANED SO AS TO BE FREE OF ALL FOREIGN MATTER WITHIN ONE INCH MINIMUM OF THE WELD LINE. ERECTOR SHALL TOUCH-UP ALL FIELD WELDS AND ABRASIONS WITH SHOP PRIMER FURNISHED BY THE FABRICATOR.			
2. SEISMIC IMPORTANCE FACTOR (Is).....1.0							
3. S.....171 kg							
4. S.....75 kg							
5. SITE CLASS.....D (REPORT)							
6. SPECTRAL RESPONSE COEFFICIENTS							
a. SDS.....0.182							
b. SD.....0.121							
7. SEISMIC DESIGN CATEGORY.....B							
8. BASIC SEISMIC-FORCE RESISTING SYSTEM.....INTERMEDIATE REINFORCED MASONRY SHEAR WALLS							
9. SEISMIC RESPONSE COEFFICIENT (Cs).....0.052							
10. RESPONSE MODIFICATION FACTOR (R).....1.0							
11. DESIGN BASE SHEAR.....0.052 W							
12. ANALYSIS PROCEDURE.....EQUIVALENT LATERAL FORCE							
15. GENERAL REQUIREMENTS							
A. VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO BEGINNING WORK OR FABRICATING MATERIALS. NOTIFY STRUCTURAL ENGINEER OF RECORD (SER) OF ANY DISCREPANCIES BEFORE PROCEEDING WITH ANY PHASE OF WORK.							
B. VERIFY THE LOCATION OF CHASES, INSERTS, OPENINGS, SLEEVES, FINISHES, DEPRESSIONS, PADS, AND WALL OPENINGS.							
C. DO NOT SCALE DRAWINGS FOR THE PURPOSE OF ESTABLISHING DIMENSIONS.							
D. DETAILS LABELED "TYPICAL DETAILS" ON DRAWINGS APPLY TO SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. SUCH DETAILS APPLY WHETHER OR NOT DETAILS ARE REFERENCED AT EACH LOCATION. NOTIFY SER OF CONFLICT REGARDING APPLICABILITY OF "TYPICAL DETAILS".							
E. DO NOT LOAD THE SLAB ON GRADE OR SUPPORTED SLAB WITH ERECTION CRANES OR ERECTION EQUIPMENT. THE SLABS HAVE NOT BEEN DESIGNED FOR CRANE LOADS AND WILL REQUIRE AN INCREASE IN THICKNESS AND/OR REINFORCEMENT. OBTAIN SER APPROVAL ON PROPOSED CRANE SUPPORT PLAN FOR SLABS PRIOR TO COMMENCING WORK.							
F. DO NOT STORE OR STACK CONSTRUCTION MATERIALS ON POURED OR ERECTED FLOORS/ROOFS IN EXCESS OF 80 PERCENT OF LIVE LOAD. GENERAL CONTRACTOR SHALL ENSURE THAT ALL SUB-CONTRACTORS ARE INFORMED OF LOADING RESTRICTIONS. AVOID IMPACT WHEN PLACING MATERIALS ON POURED OR ERECTED FLOORS OR ROOF.							
G. THE CONTRACT STRUCTURAL DOCUMENTS REPRESENT THE FINISHED STRUCTURE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION. PROVIDE ALL MEASURES REQUIRED TO PROTECT THE STRUCTURE, WORKMEN, AND OTHER PERSONS DURING CONSTRUCTION, INCLUDING BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR THE BUILDING, FORMS AND SCAFFOLDING, SHORING OF RETAINING WALLS AND OTHER TEMPORARY SUPPORTS AS REQUIRED. COMPLY WITH APPLICABLE REQUIREMENTS OF OSHA AND OTHER GOVERNING BODIES HAVING JURISDICTION AT THE SITE.							
H. PRINCIPAL OPENINGS THROUGH THE FRAMING ARE SHOWN ON DRAWINGS. EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR THE REQUIRED OPENINGS AND PROVIDE FOR REQUIRED OPENINGS WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT. VERIFY SIZE AND LOCATION OF OPENINGS WITH THE MECHANICAL CONTRACTOR. DEVIATIONS FROM THE OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS MUST BE APPROVED BY THE SER PRIOR TO IMPLEMENTING THE CHANGES.							
J. LOADINGS FOR MECHANICAL EQUIPMENT ARE BASED ON THE UNITS SHOWN ON THE MECHANICAL DRAWINGS. ANY CHANGES IN TYPE, SIZE, OR NUMBER OF PIECES OF EQUIPMENT SHALL BE REPORTED TO THE SER FOR VERIFICATION OF THE ADEQUACY OF SUPPORTING MEMBERS PRIOR TO THE PLACEMENT OF SUCH EQUIPMENT.							
K. SEE ARCHITECTURAL DRAWINGS FOR ELEVATIONS NOT SHOWN AND FOR EXACT LOCATIONS OF ALL SLAB DEPRESSIONS. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL SECTIONS WITH ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCY TO THE SER PRIOR TO FABRICATING OR INSTALLING STRUCTURAL MEMBERS.							
16. SPECIAL INSPECTIONS							
A. SPECIAL INSPECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH THE GOVERNING BUILDING CODE AND THE STATEMENT OF REQUIRED SPECIAL INSPECTIONS PREPARED BY THE DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.							
B. WHERE SPECIAL INSPECTION REQUIREMENTS DUPLICATE THE REQUIREMENTS OF OTHER SPECIFIED TESTING, DUPLICATE INSPECTIONS SHALL NOT BE REQUIRED.							

BASED ON 8-BAY SINGLE SIDED PROTO. DATED APRIL 01 2022

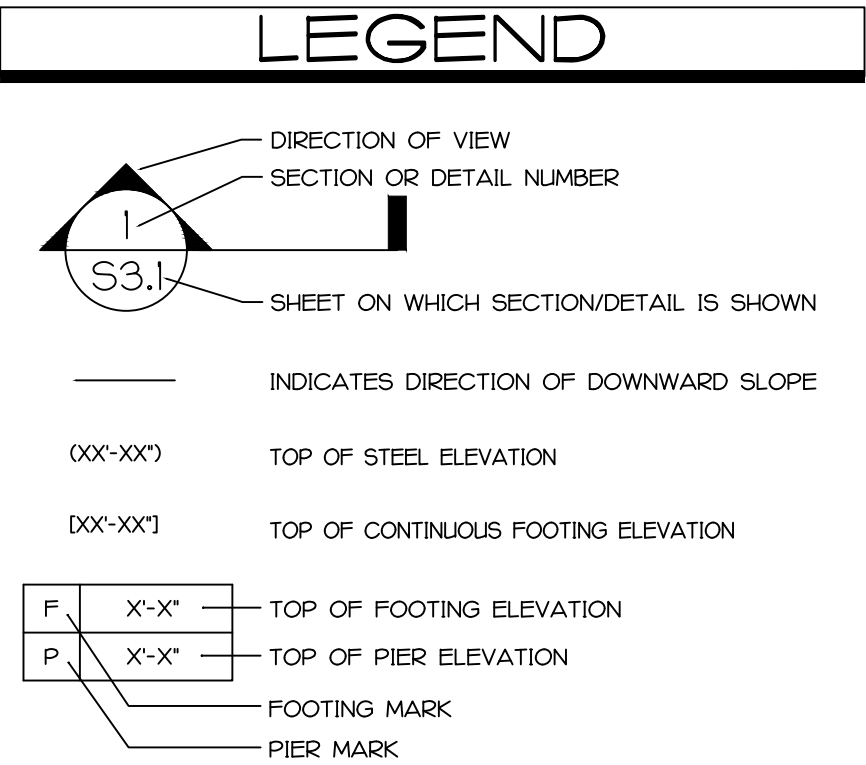
CONCRETE MIX A				
28 DAY COMPRESSIVE STRENGTH (f _c).....3,000 PSI				
LOCATION: FOOTINGS				
	SEVERITY	CLASS	CONDITION	
F FREEZING AND THAWING	N/A	FO	CONCRETE NOT EXPOSED TO FREEZING-AND-THAWING CYCLES	
S SULFATE	N/A	SO	WATER -SOLUBLE SULFATE (SO4) IN SOIL, PERCENT BY MASS	DISSOLVED SULFATE (SO4) IN WATER, PPM
			SO4 < 0.10	SO4 < 150
W IN CONTACT WITH WATER	N/A	WO	CONCRETE IN CONTACT WITH WATER AND LOW PERMEABILITY IS NOT REQUIRED	
C CORROSION PROTECTION OF REINFORCEMENT	N/A	CI	CONCRETE EXPOSED TO MOISTURE BUT NOT TO EXTERNAL SOURCES OF CHLORIDES	
NOTES:				
1. REFERENCE ACI 318-14 TABLE 4.2.1-EXPOSURE CATEGORIES AND CLASSES				
CONCRETE TESTING:				
1. PERFORM SLUMP, AIR CONTENT, COMPRESSION, AND CONCRETE TEMPERATURE TESTING IN ACCORDANCE WITH THE PROJECT STATEMENT OF SPECIAL INSPECTION				
2. OBTAIN NO FEWER THAN TWO COMPOSITE SAMPLES FOR CONCRETE MIX A.				
3. OBTAIN AT LEAST ONE COMPOSITE SAMPLE FOR EACH DAY CONCRETE IS PLACED.				

CONCRETE MIX B				
28 DAY COMPRESSIVE STRENGTH (f _c).....4,500 PSI				
LOCATION: PIERS				
	SEVERITY	CLASS	CONDITION	
F FREEZING AND THAWING	N/A	F2	CONCRETE EXPOSED TO FREEZING-AND-THAWING CYCLES WITH FREQUENT EXPOSURE TO WATER	
S SULFATE	N/A	SO	WATER -SOLUBLE SULFATE (SO4) IN SOIL, PERCENT BY MASS	DISSOLVED SULFATE (SO4) IN WATER, PPM
			SO4 < 0.10	SO4 < 150
W IN CONTACT WITH WATER	N/A	WO	CONCRETE IN CONTACT WITH WATER AND LOW PERMEABILITY IS REQUIRED	
C CORROSION PROTECTION OF REINFORCEMENT	N/A	CI	CONCRETE EXPOSED TO MOISTURE, BUT NOT TO EXTERNAL SOURCES OF CHLORIDES	
NOTES: 1. REFERENCE ACI 318-14 TABLE 4.2.1-EXPOSURE CATEGORIES AND CLASSES				
CONCRETE TESTING: 1. PERFORM SLUMP, AIR CONTENT, COMPRESSION, AND CONCRETE TEMPERATURE TESTING IN ACCORDANCE WITH THE PROJECT STATEMENT OF SPECIAL INSPECTION 2. OBTAIN NO FEWER THAN TWO COMPOSITE SAMPLES FOR CONCRETE MIX B. 3. OBTAIN AT LEAST ONE COMPOSITE SAMPLE FOR EACH DAY CONCRETE IS PLACED.				

CONCRETE MIX C				
28 DAY COMPRESSIVE STRENGTH (fc).....4,000 PSI				
LOCATION: INTERIOR SLABS				
	SEVERITY	CLASS	CONDITION	
F FREEZING AND THAWING	N/A	FO	CONCRETE NOT EXPOSED TO FREEZING-AND-THAWING CYCLES	
S SULFATE	N/A	SO	WATER -SOLUBLE SULFATE (SO4) IN SOIL, PERCENT BY MASS	DISSOLVED SULFATE (SO4) IN WATER, PPM
			SO4 < 0.10	SO4 < 150
W IN CONTACT WITH WATER	N/A	WO	CONCRETE IN CONTACT WITH WATER AND LOW PERMEABILITY IS NOT REQUIRED	
C CORROSION PROTECTION OF REINFORCEMENT	N/A	CI	CONCRETE EXPOSED TO MOISTURE BUT NOT TO EXTERNAL SOURCES OF CHLORIDES	
NOTES:				
1. REFERENCE ACI 318-14 TABLE 4.2.1-EXPOSURE CATEGORIES AND CLASSES				
CONCRETE TESTING:				
1. PERFORM SLUMP, AIR CONTENT, COMPRESSION, AND CONCRETE TEMPERATURE TESTING IN ACCORDANCE WITH THE PROJECT STATEMENT OF SPECIAL INSPECTION				
2. OBTAIN NO FEWER THAN TWO COMPOSITE SAMPLES FOR CONCRETE MIX C.				
3. OBTAIN AT LEAST ONE COMPOSITE SAMPLE FOR EACH DAY CONCRETE IS PLACED.				

TENSION BAR DEVELOPMENT LENGTH									
BAR SIZE	GRADE 60 BAR	3000 PSI		4000 PSI		4500 PSI		5000 PSI	
	BAR SPACING:	(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia
#3 (3/8")	TOP BARS:	22	33	19	28	18	27	17	25
	OTHER BARS:	17	25	15	22	14	21	13	20
#4 (1/2")	TOP BARS:	29	43	25	37	24	35	23	34
	OTHER BARS:	22	33	19	29	18	27	17	26
#5 (5/8")	TOP BARS:	36	54	31	47	30	44	28	42
	OTHER BARS:	28	42	24	36	23	34	22	32
#6 (3/4")	TOP BARS:	43	65	37	56	35	53	34	50
	OTHER BARS:	33	50	29	43	27	41	26	39
#7 (7/8")	TOP BARS:	63	94	54	81	51	77	49	73
	OTHER BARS:	48	72	42	63	40	59	38	56
#8 (1")	TOP BARS:	72	107	62	93	59	88	56	83
	OTHER BARS:	55	83	48	72	45	68	43	64
#9 (1.125")	TOP BARS:	81	121	70	105	66	99	63	94
	OTHER BARS:	62	93	54	81	51	76	48	72
#10 (1.270")	TOP BARS:	91	136	79	118	74	111	71	106
	OTHER BARS:	70	105	61	91	57	86	54	81
#11 (1.410")	TOP BARS:	101	151	87	131	82	123	78	117
	OTHER BARS:	78	116	67	101	64	95	60	90
TABLE NOTES: 1. LENGTHS APPLY TO UNCOATED REINFORCEMENT IN NORMAL WEIGHT CONCRETE ONLY. 2. TOP BARS REFERS TO HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE.									

TENSION BAR LAP SPLICE LENGTH FOR CLASS B SPLICES									
BAR SIZE	GRADE 60 BAR	3000 PSI		4000 PSI		4500 PSI		5000 PSI	
	BAR SPACING:	(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia	(3) Bar Dia or More	Less Than (3) Bar Dia
#3 (3/8")	TOP BARS:	28	42	25	37	23	35	22	33
	OTHER BARS:	22	33	19	28	18	27	17	25
#4 (1/2")	TOP BARS:	38	56	33	49	31	46	29	44
	OTHER BARS:	29	43	25	37	24	35	23	34
#5 (5/8")	TOP BARS:	47	70	41	61	38	57	36	54
	OTHER BARS:	36	54	31	47	30	44	28	42
#6 (3/4")	TOP BARS:	56	84	49	73	46	69	44	65
	OTHER BARS:	43	65	37	56	35	53	34	50
#7 (7/8")	TOP BARS:	81	122	71	106	67	100	63	95
	OTHER BARS:	63	94	54	81	51	77	49	73
#8 (1")	TOP BARS:	93	139	81	121	76	114	72	108
	OTHER BARS:	72	107	62	93	59	88	56	83
#9 (1.125")	TOP BARS:	105	157	91	136	86	128	81	122
	OTHER BARS:	81	121	70	105	66	99	63	94
#10 (1.270")	TOP BARS:	118	177	102	153	96	144	92	137
	OTHER BARS:	91	136	79	118	74	111	71	106
#11 (1.410")	TOP BARS:	131	196	114	170	107	160	102	152
	OTHER BARS:	101	151	87	131	82	123	78	117
TABLE NOTES: 1. LENGTHS APPLY TO UNCOATED REINFORCEMENT IN NORMAL WEIGHT CONCRETE ONLY. 2. TOP BARS REFERS TO HORIZONTAL REINFORCEMENT SO PLACED THAT MORE THAN 12 INCHES OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE DEVELOPMENT LENGTH OR SPLICE.									



REINFORCING NOTES	
1.	UNLESS NOTED OTHERWISE MAINTAIN THE FOLLOWING CONCRETE COVER FOR REINFORCEMENT: a. CONCRETE CAST AGAINST EARTH.....3 INCHES b. CONCRETE EXPOSED TO THE WEATHER: #5 AND SMALLER BARS.....1 1/2 INCHES #6 AND LARGER BARS.....2 INCHES c. CONDITIONS OTHER THAN ABOVE #5 AND SMALLER BARS.....3/4 INCHES #6 AND LARGER BARS.....1 1/2 INCHES
2.	PROVIDE STANDARD 90° HOOKS IN ACCORDANCE WITH ACI318 UNLESS NOTED OTHERWISE.
3.	PROVIDE CLASS "B" REINFORCEMENT SPLICES FOR CONTINUOUS REINFORCEMENT. REINFORCEMENT SPLICES AND DEVELOPMENT LENGTHS SHALL BE IN ACCORDANCE WITH ACI318-08 AND THE TABLES PROVIDED WITH THESE DRAWINGS. WHERE THERE IS A CONFLICT, THE MORE STRINGENT REQUIREMENT SHALL APPLY. SPLICES FOR WELDED WIRE FABRIC SHALL BE TWO (2) INCHES IN ADDITION TO ONE SPACING OF CROSS WIRES.
4.	PROVIDE CONTINUOUS HORIZONTAL WALL REINFORCEMENT WITH 90° BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL REINFORCEMENT DETAILS.
5.	ALL RE-ENTRANT CORNERS FOR SLAB-ON-GRADE CONSTRUCTION SHALL BE REINFORCED WITH ONE (1) #4 BY 4'-0" LONG (MIN) AT 45° FROM THE SLAB EDGES AND AT SLAB MID-DEPTH.
6.	WHERE REQUIRED AND UNLESS NOTED OTHERWISE, PROVIDE DOWELS TO MATCH SIZE AND SPACING OF MAIN REINFORCEMENT.
7.	DO NOT WELD OR BEND REINFORCEMENT IN THE FIELD UNLESS SPECIFICALLY SHOWN OR APPROVED BY STRUCTURAL ENGINEER.

REVISIONS

BY

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Date: 11. 11. 22

Scale: AS NOTED

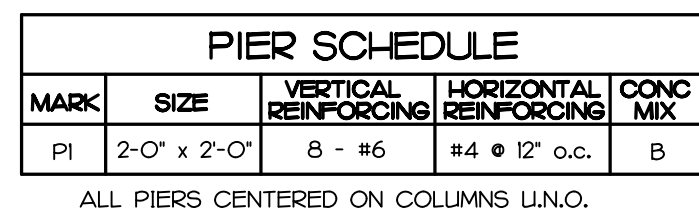
Project Mgr: MD

Drawn: CAD

Job: 4601

Sheet

S1.2



COLUMN SCHEDULE			
MARK	SHAPE	BASE PLATE	ANCH. BOLTS
C1	W8x31	BP1	AB1
C2	HSS5x5x1/4	BP2	AB1



BASE PLATE SCHEDULE					
BASE PLATE	TYPE	SIZE			ANCHOR ROD
		T	B	N	
BP1	A	1"	15"	15"	(4) AB2
BP2	B	3/4"	11"	11"	(4) AB1

ANCHOR ROD SCHEDULE						
MARK	DIAMETER D (IN)	EMBEDMENT E (IN)	PROJECTION P (IN)	WASHER SIZE (IN)	WASHER THICKNESS (IN)	REMARKS
AB1	3/4"	12"	5" MIN.	2" DIA OR SQ	1/4"	-
AB2	1"	12"	5" MIN.	3" DIA OR SQ	3/8"	-


$$\frac{3}{16}'' = 1'-O''$$

FOUNDATION SHEET NOTES:

FINISHED FLOOR ELEVATION IS 0.00' UNLESS NOTED OTHERWISE

GENERAL CONTRACTOR COORDINATING
WITH FINAL ARCH. FLOOR PLANS.

BOF — INDICATES BOTTOM OF FOOTING
BW — INDICATES BOTTOM OF WALL
-C- — INDICATES SLAB CONTROL JOINT

TW
TOP
FFE
EOS
FOE
FOM
O.H.


- INDICATES TOP OF WALL
- INDICATES TOP OF FOOTING
- INDICATES FINISHED FLOOR ELEVATION
- INDICATES EDGE OF SLAB
- INDICATES FACE OF BUILDING
- INDICATES FACE OF MASONRY
- INDICATES OPPOSITE HAND

- INDICATES BORING LOCATIONS - SEE REPORT



INDICATES COLUMN CENTERLINE
INDICATES 8" SLAB
PROVIDE CONSTRUCTION JOINTS
AROUND 8" SLAB NOTED ON PLAN
INDICATES 4" SLAB
PROVIDE CONSTRUCTION JOINTS
AROUND 4" SLAB NOTED ON PLAN

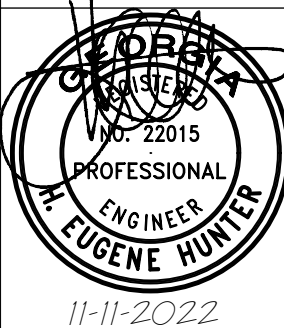
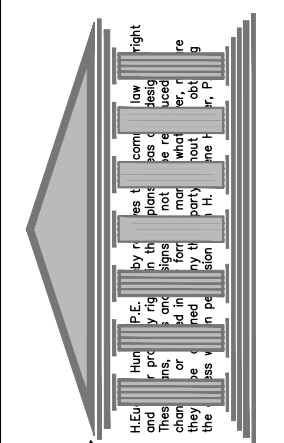
SLAB PLAN NOTES:

1. RE: SHEET SL1 AND SL2 FOR GENERAL NOTES.
2. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL PLANS AND DETAILS.
3. INTERIOR SLAB TO HAVE POLISHED FINISH.
4. COORDINATE ALL CONDUIT EMBEDMENTS AND PLUMBING STUB-UP LOCATIONS WITH MECHANICAL AND ELECTRICAL CONTRACTORS.
5. COORDINATE WITH ELECTRICAL CONTRACTOR AND ELECTRICAL
6. ONE-LINE DIAGRAM FOR REINFORCEMENT BOLD TO GROUNDING SYSTEM.
7. ARCHITECTURAL
8. 4" CONCRETE SLAB REINFORCING: W/ (1) LAYER WWR 6x6-W2.9x2.9 @
MID-DEPTH, CONCRETE MIX C
9. 6" CONCRETE SLAB REINFORCING: W/ (1) LAYER WWR 6x6-W4.0x4.0 @
MID-DEPTH, CONCRETE MIX C
10. 8" CONCRETE SLAB REINFORCING: #4 @ 12" OC EACH WAY, CONCRETE MIX C
11. 10" HIGH CMU WALL TO BE BUILT AFTER STORAGE RACK SYSTEM IS
INSTALLED

NOTES:

1. CONTRACTOR TO ENSURE NO CRACKS OR CTJ ARE LOCATED WITHIN 6' OF LIFT INSTALLATION ANCHORS. CJ TO BE LOCATED IN ACCORDANCE WITH THE PLAN AND CONSTRUCTED PER 4/531
2. CONTRACTOR TO CONTACT FLOOR POLISHING VENDOR ONE WEEK PRIOR TO POURING FLOOR. COORDINATE WITH MAVIS TIRE NATIONAL ACCOUNT VENDOR: CPR CONCRETE, ATTN. BRENT SABINO. CELL: 865-630-1101. EMAIL: BRENT.SABINO@CPRCONCRETE.COM
3. SLOPE ALL GRADES AWAY FROM BUILDING.

BASED ON 8-BAY SINGLE SIDED PROTO. DATED APRIL 01 2022



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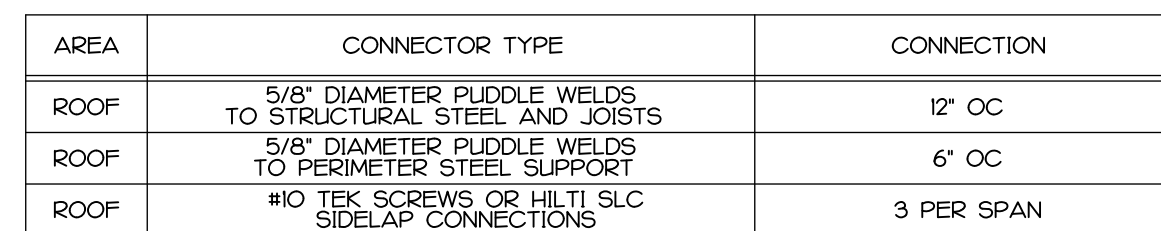
Project Mgr: MD

Drawn: CAD

Job: 4589

Sheet

S2



LINTEL SCHEDULE		
MARK	DETAIL	DESCRIPTION
LI		10" x 8" CMU LINTEL W/ (2) #5 BAR

NOTE: PROVIDE 8" MIN LENGTH OF BEARING AT EACH
END OF ALL MASONRY LINTELS. MORTAR LINTEL INTO
PLACE SOLID, ABOVE AND BELOW BEARING.

FRAMING PLAN NOTES:

- FRAMING PLAN NOTES:**
1. RE: SHEET SI - SL2 FOR GENERAL NOTES.
 2. COORDINATE ALL DIMENSIONS WITH ARCHITECTURAL PLANS AND DETAILS.
 3. COORDINATE ALL DIMENSIONS AND LOCATION OF MECHANICAL EQUIPMENT WITH MECHANICAL PLANS AND DETAILS AND THE EQUIPMENT MANUFACTURER.
 4. INDICATES ROOF DECK SPAN.
 5. ROOF DECK TO BE 1 1/2" TYPE B 20 GA DECK, GALVANIZED W/ G60 ZINC COATING.
 6. JOIST SPACING TO BE 24" O.C. OF SH20P PRIMED JOISTS. RE: SPECS SECTION 052100 FOR MORE DETAILS ON THE SH20P PRIMER & PAINT.
 7. GIRDER LOADS INCLUDE RTU WEIGHT.

WIND NET UPLIFT (PSF)			
ROOF DECK		ROOF JOISTS	
TYPICAL	WITHIN 5'-3" OF PERIMETER	TYPICAL	WITHIN 5'-3" OF PERIMETER
19	46	11	14

NOTES

- NOTES:**
1. FOR JOISTS: EFFECTIVE WIND AREA = 100 SQ FT.
 2. FOR JOISTS: ROOF NET UPLIFT VALUES ARE DETERMINED FROM ASCE 7-16 LOAD COMBINATION #7 (0.6D+0.6W). D IS TAKEN AS 10 PSF.
 3. FOR SHEATHING: EFFECTIVE WIND AREA = 10 SQ FT.
 4. FOR SHEATHING: ROOF NET UPLIFT VALUES ARE DETERMINED FROM ASCE 7-16 LOAD COMBINATION #5 (0.6W).


$$\frac{3}{16}'' = I-O''$$

SHEET NOTES:

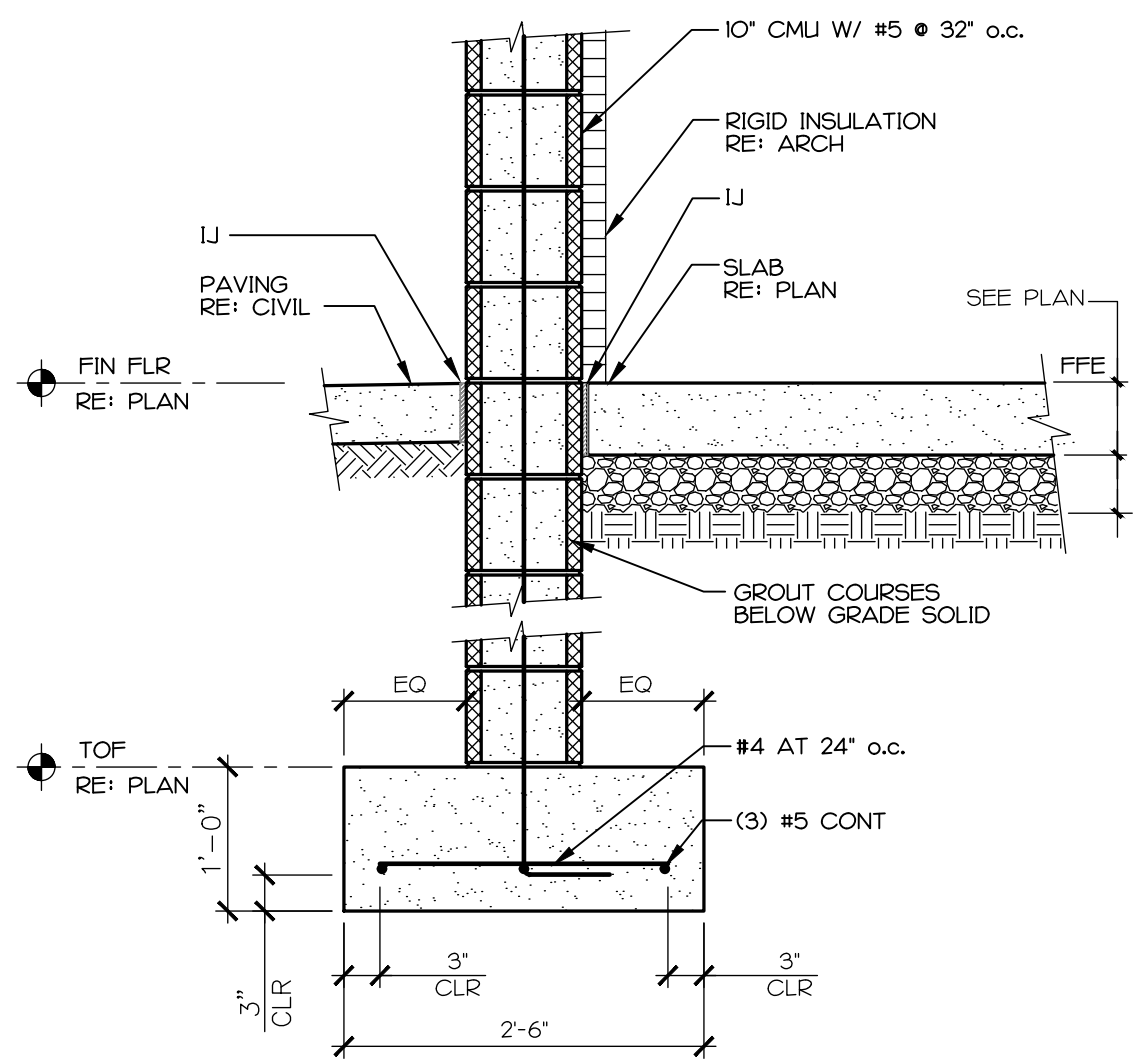
TOP OF STEEL ELEVATION (T/STL) IS MEASURED RELATIVE TO FINISHED FLOOR ELEVATION (+O'-O").

COORD. ALL MECHANICAL UNITS AND THEIR LOCATIONS WITH MECHANICAL CONTRACTOR, ARCHITECT/ENGINEER SHALL BE NOTIFIED OF CHANGES IN LOCATION, WEIGHT OR ADDITION OF ROOF SUPPORTED EQUIPMENT.

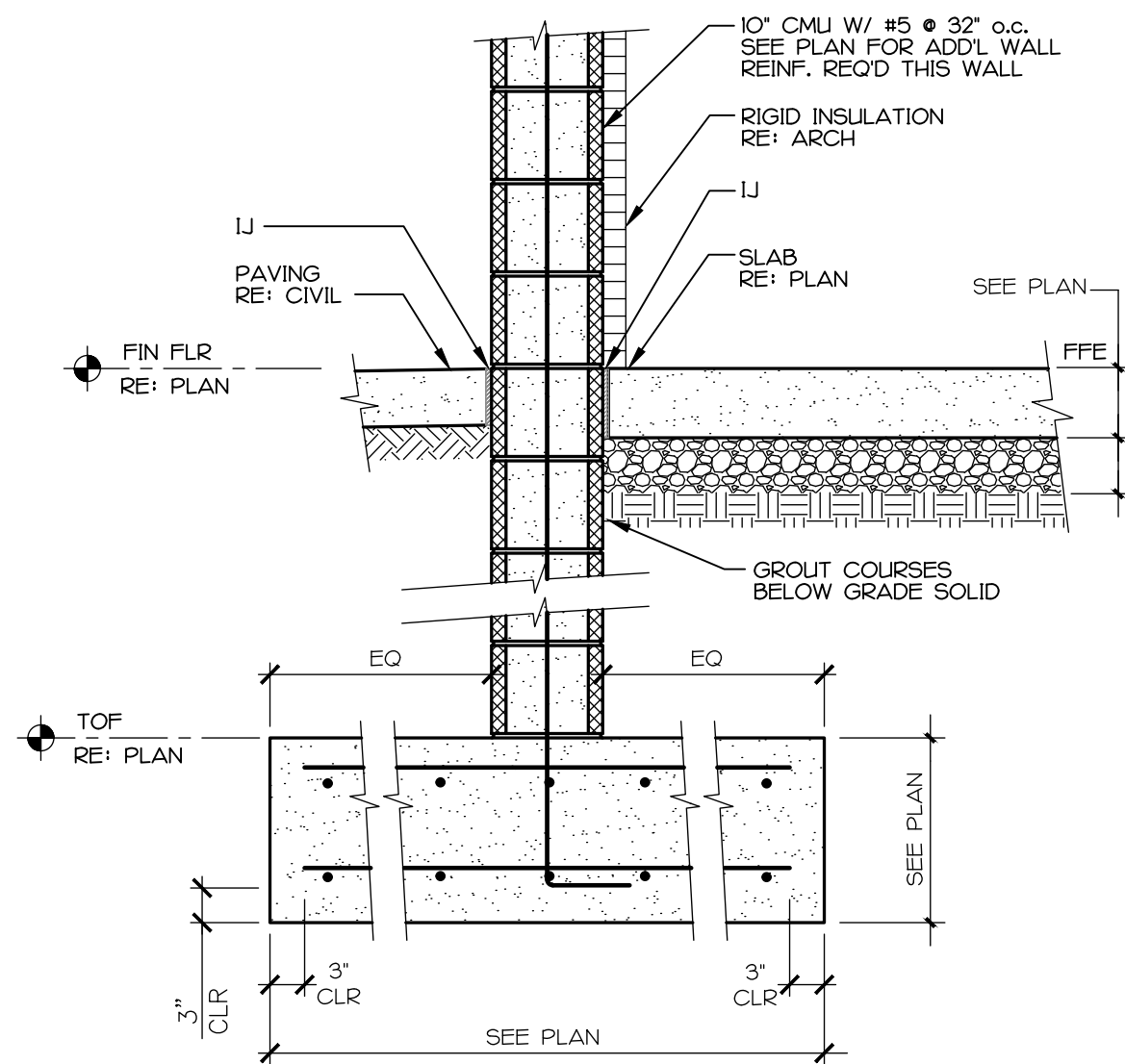
COORDINATE ALL DIMENSIONS WITH LATEST ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.

T/STL — INDICATES TOP OF STEEL
JBE — INDICATES JOIST BEARING ELEVATION
FOS — INDICATES FACE OF STUD
FOM — INDICATES FACE OF CMU
—— — LINETYPE INDICATES NON-LOAD BRG. WALL

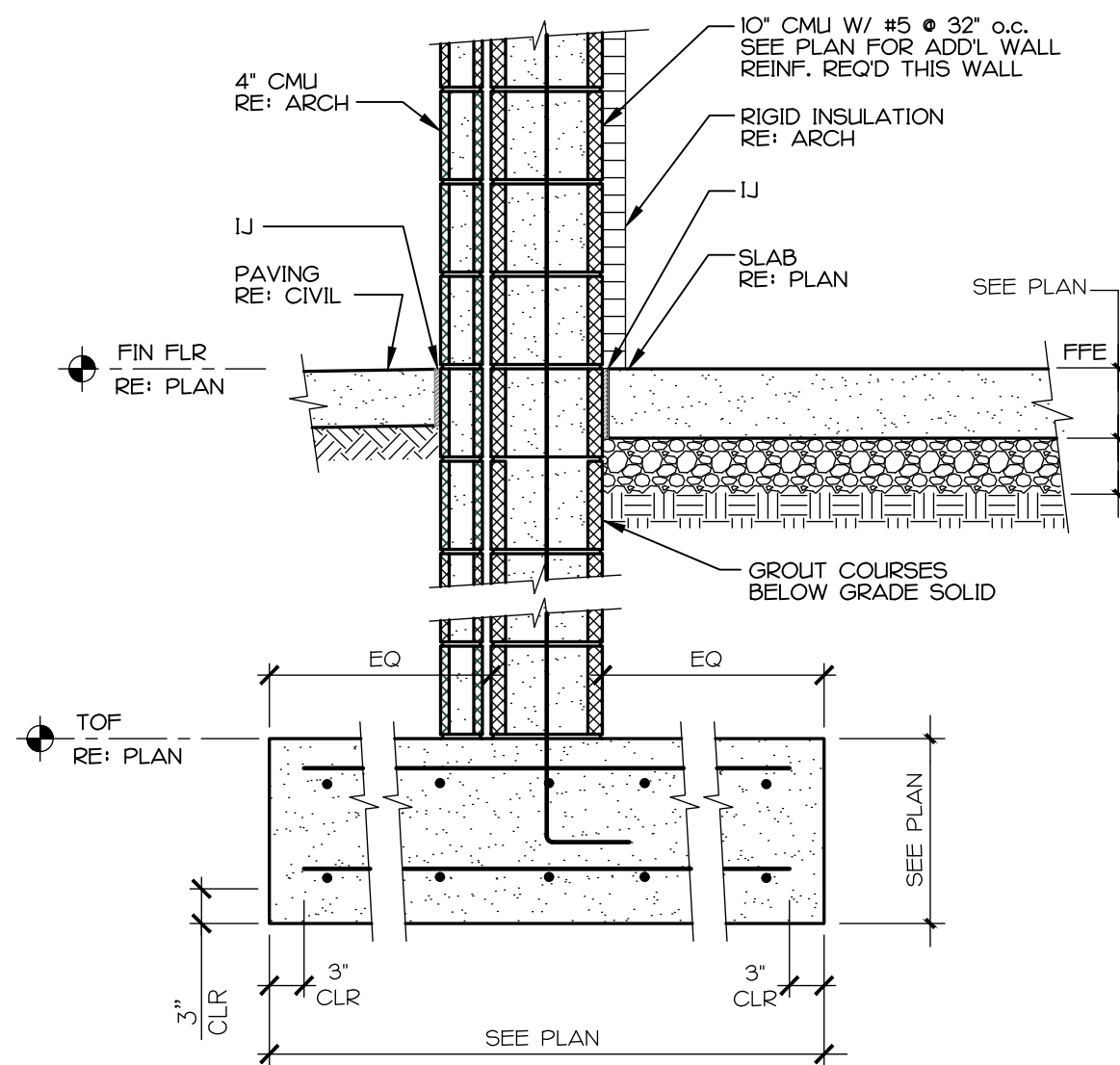
* — — INDICATES COLUMN CENTERLINE



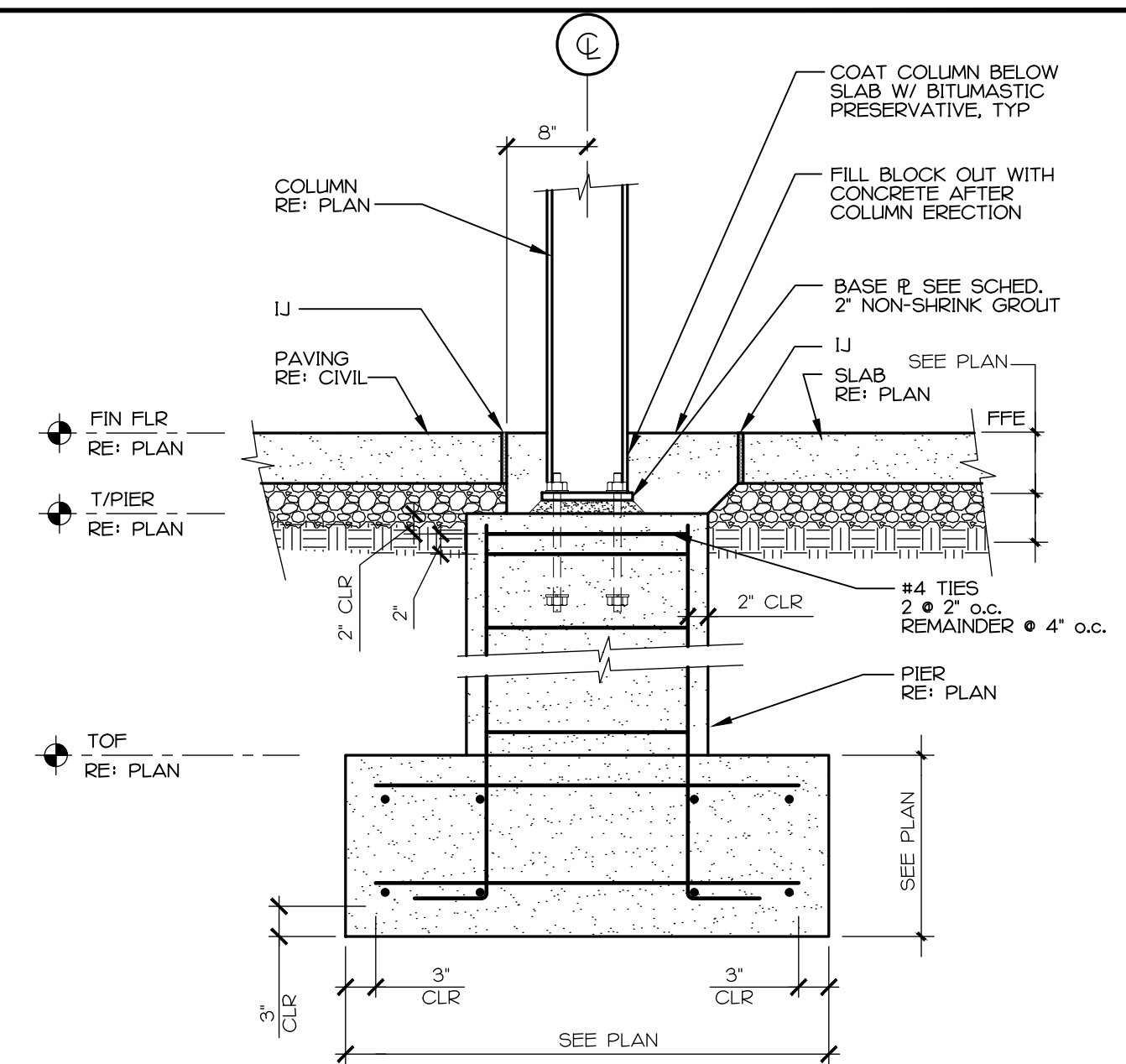
IA
S3 EXTERIOR WALL FOUNDATION SECTION
3/4" = 1'-0"



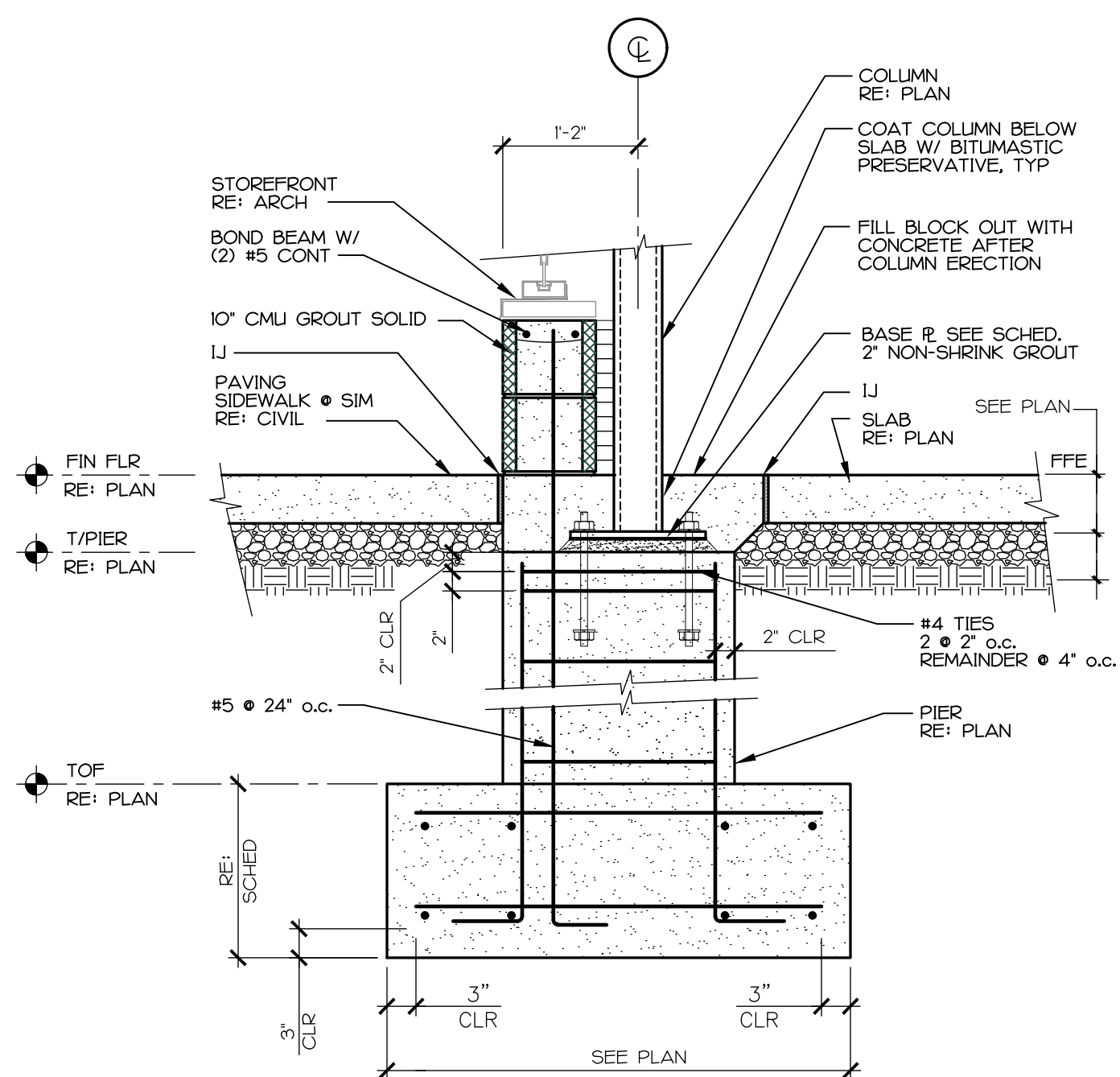
IB
S3 EXTERIOR WALL FOUNDATION SECTION
3/4" = 1'-0"



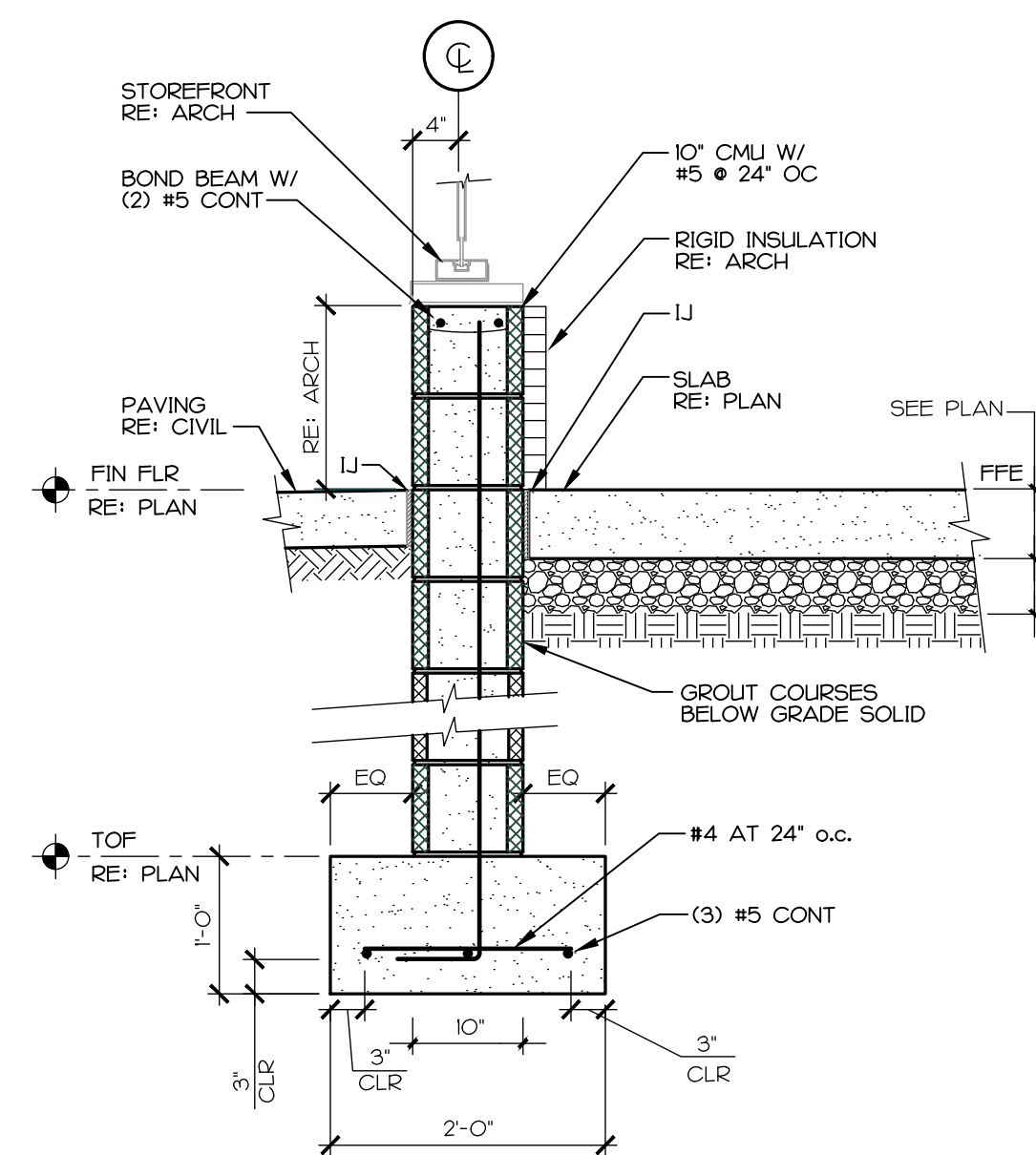
2
S3 EXTERIOR WALL FOUNDATION SECTION
3/4" = 1'-0"



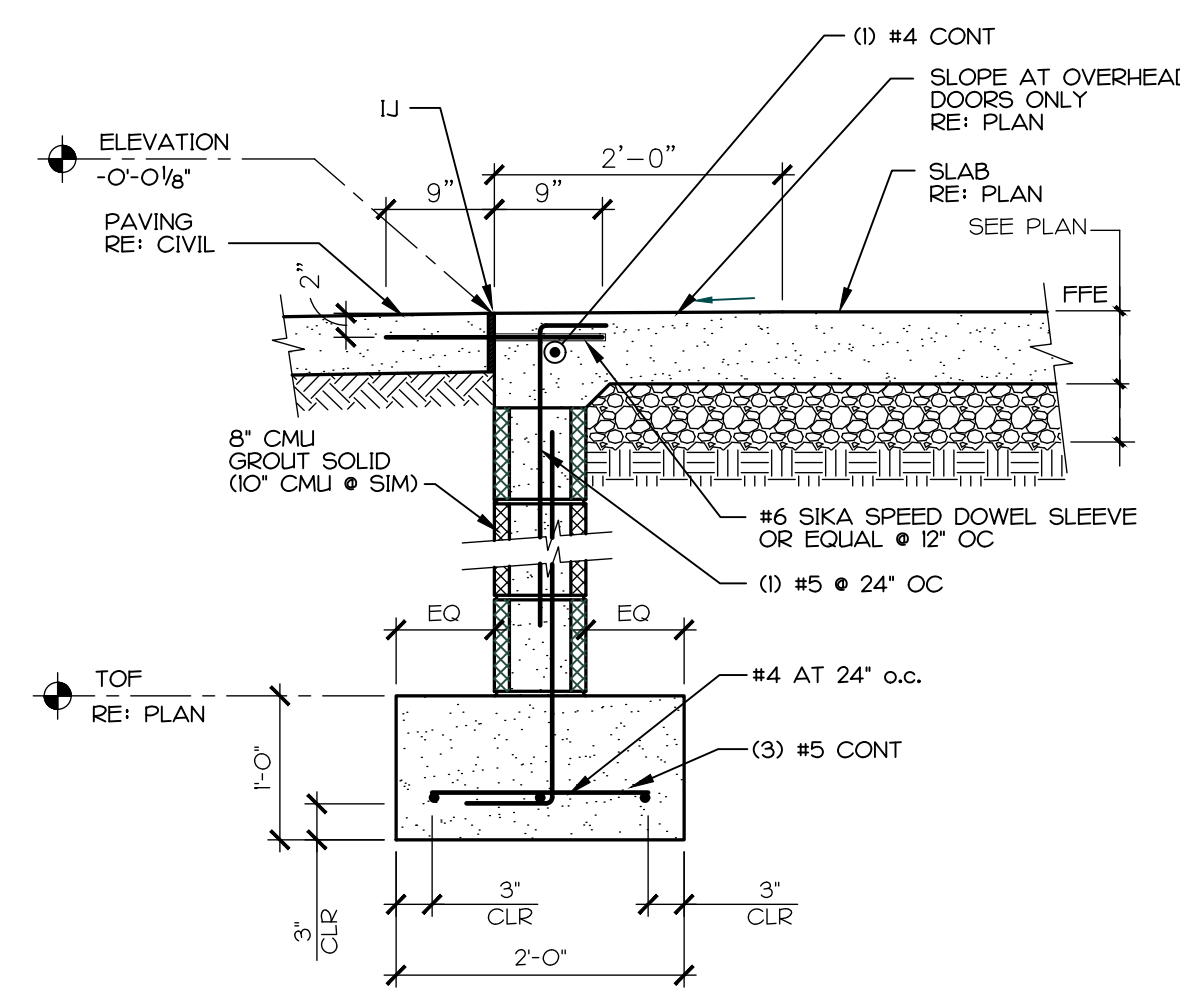
3
S3 TYP. EXTERIOR COLUMN SECTION
3/4" = 1'-0"



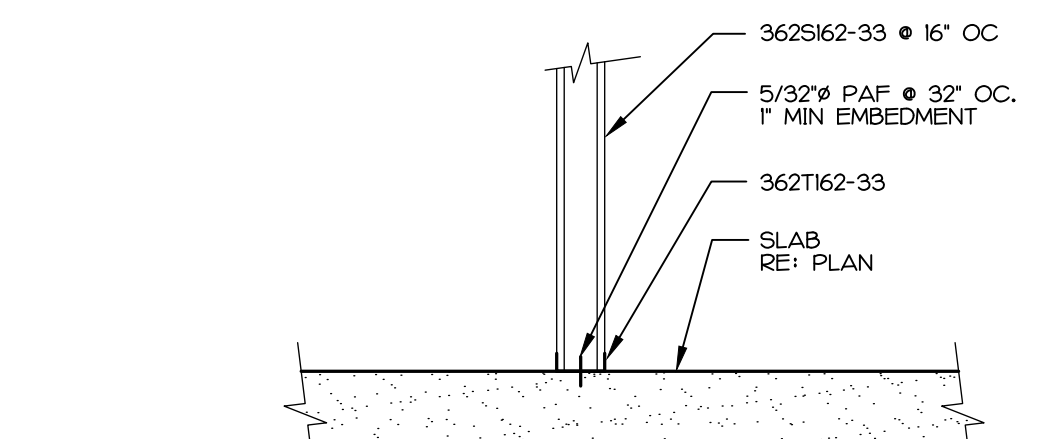
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S3 TYP. EXTERIOR COLUMN SECTION
3/4" = 1'-0"



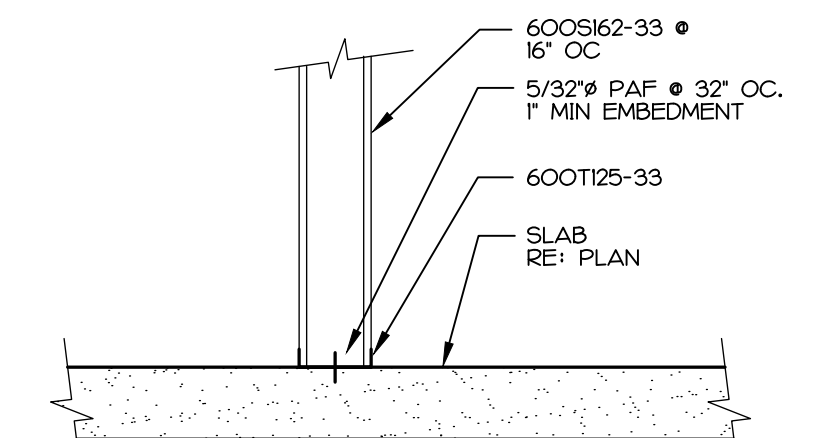
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S3 TYP. EXTERIOR WALL FDN. • SHOWROOM
3/4" = 1'-0"



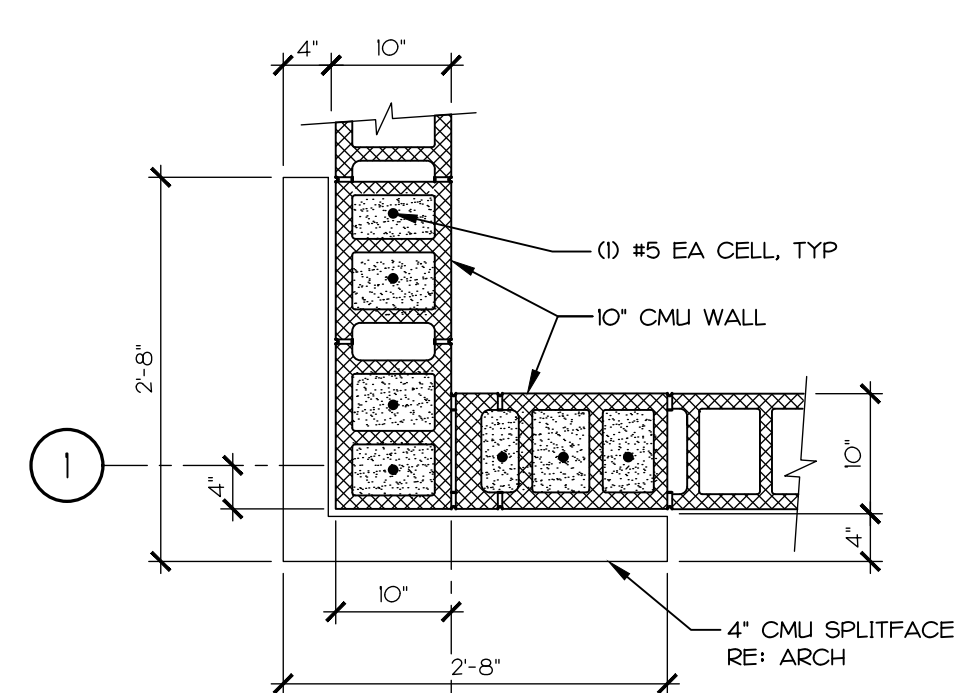
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S3 TYP. SECTION AT DOOR OPENING
3/4" = 1'-0"



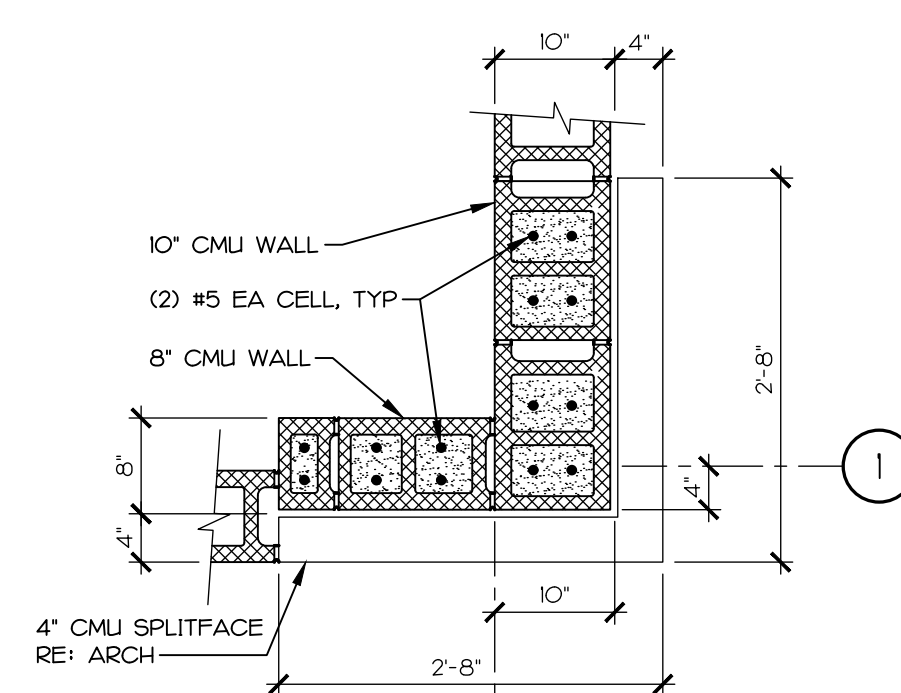
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S3 TYP. SECTION • 3 5/8" STUD WALL
3/4" = 1'-0"



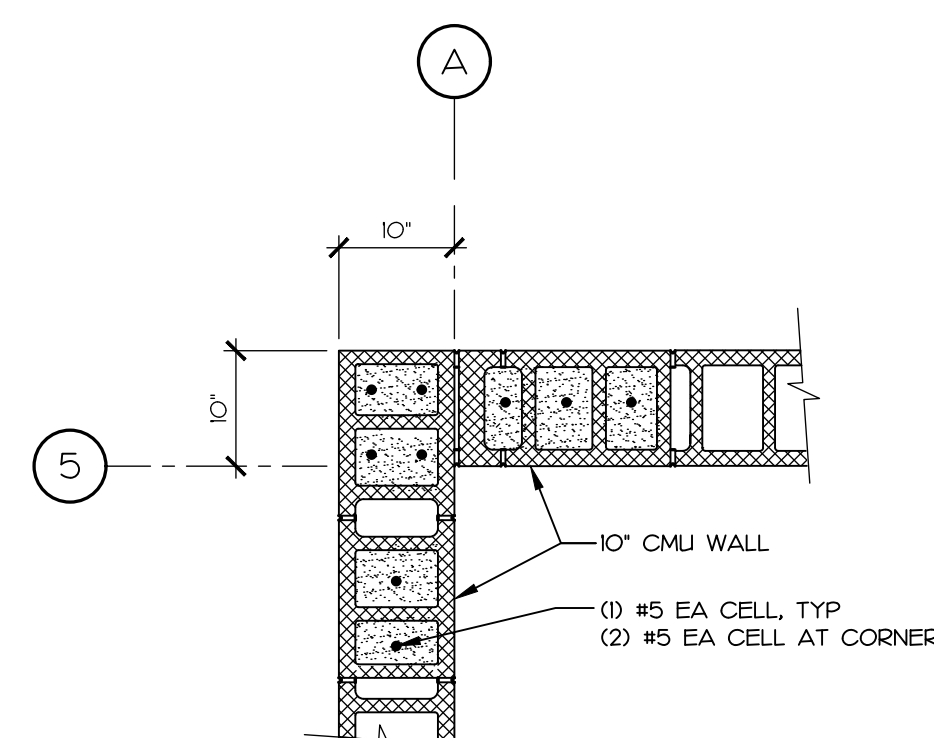
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S3 TYP. SECTION • 6" STUD WALL
3/4" = 1'-0"



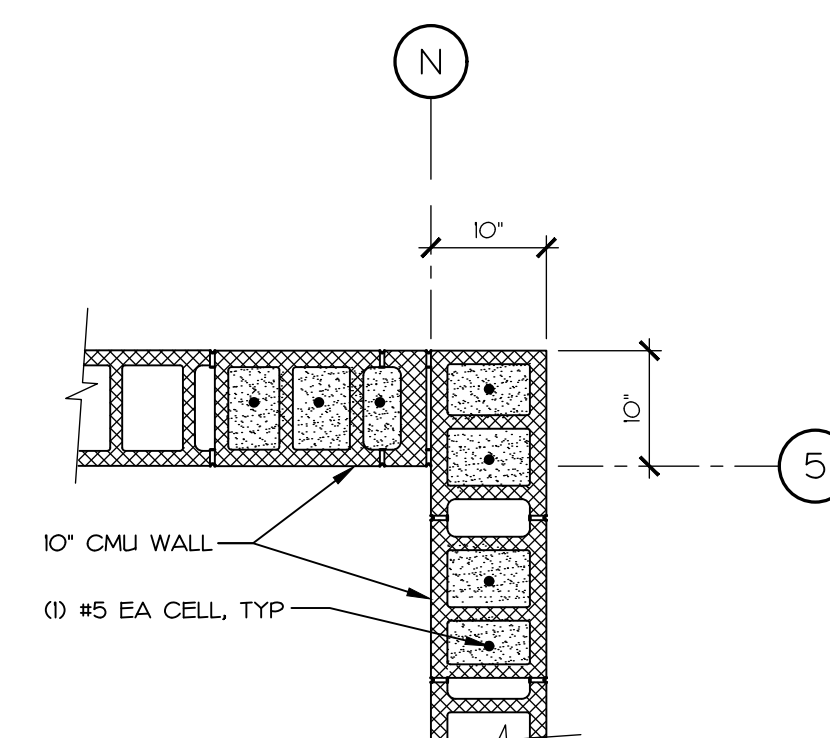
DETAIL A



DETAIL B



DETAIL C

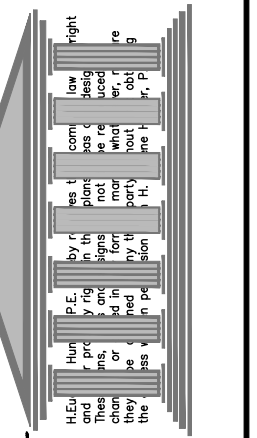


DETAIL D

9
S3 PLAN AT CMU
3/4" = 1'-0"

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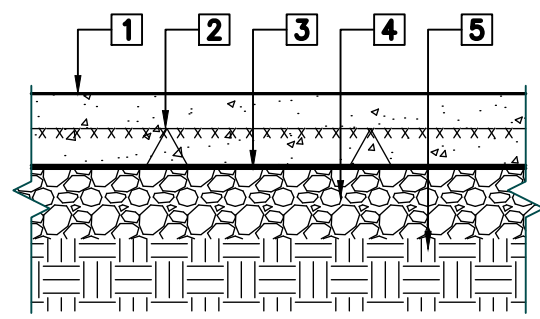
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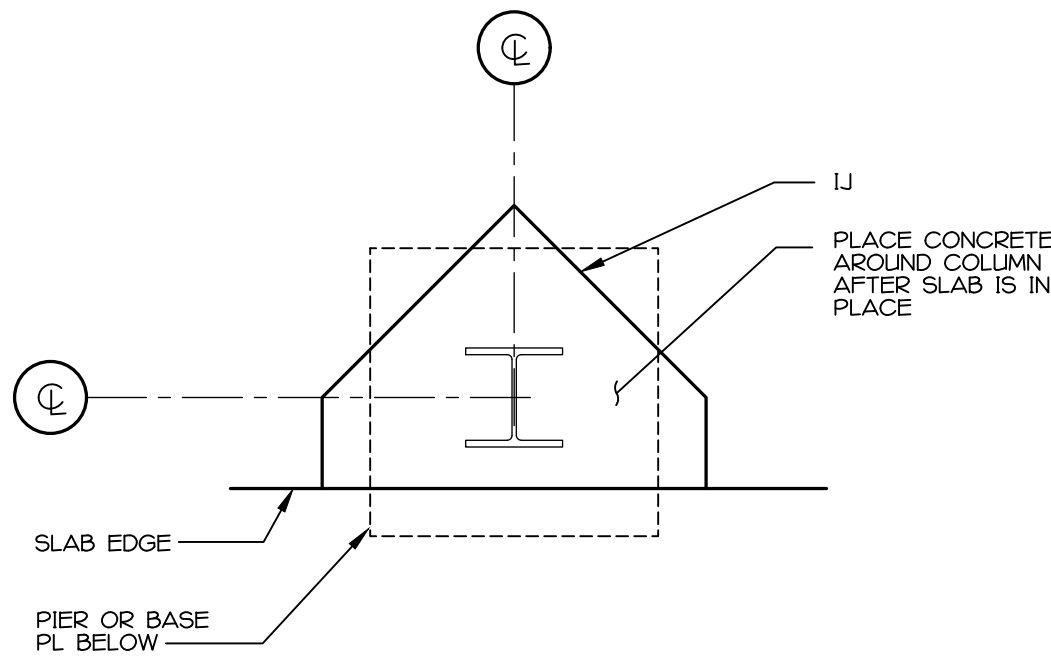
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Drawn: CAD
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S3

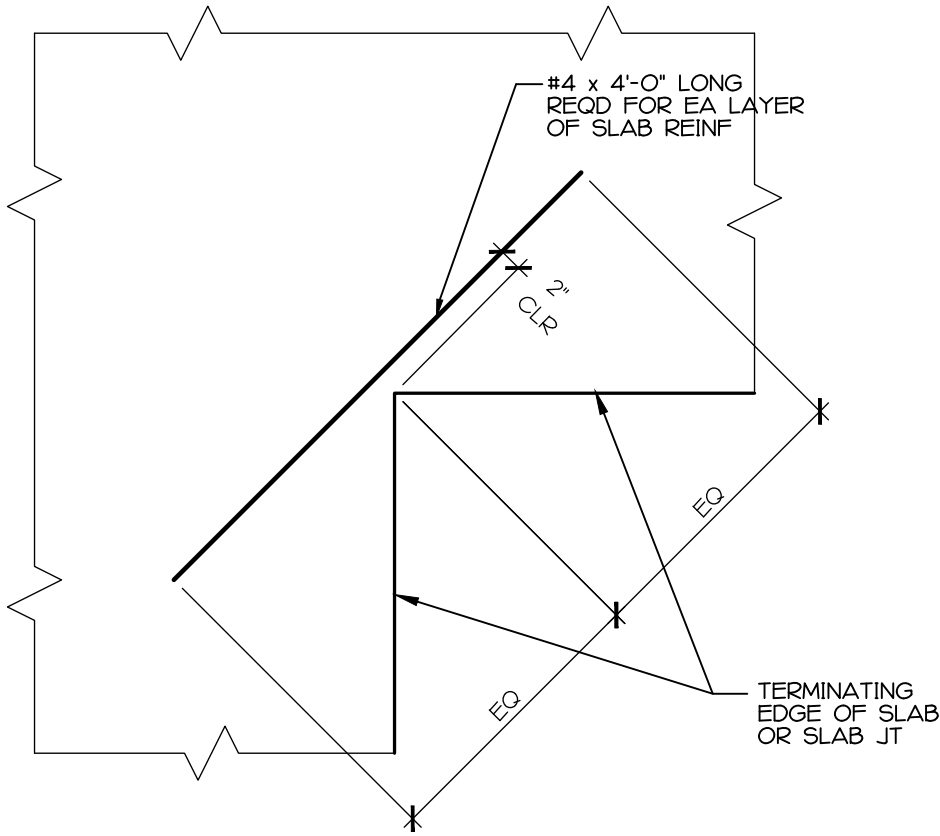


- NOTES:
1. CONCRETE SLAB, REFER TO PLAN FOR THICKNESS
 2. STEEL REINFORCEMENT, REFER TO PLAN FOR SIZE, SPACING AND LOCATION
 3. VAPOR RETARDER REFER TO STRUCTURAL NOTES. RETARDER NOT REQUIRED AT EXTERIOR SLABS UNLESS NOTED OTHERWISE
 4. GRANULAR BASE MATERIAL, REFER TO STRUCTURAL NOTES
 5. PREPARED SUBGRADE, REFER TO STRUCTURAL NOTES

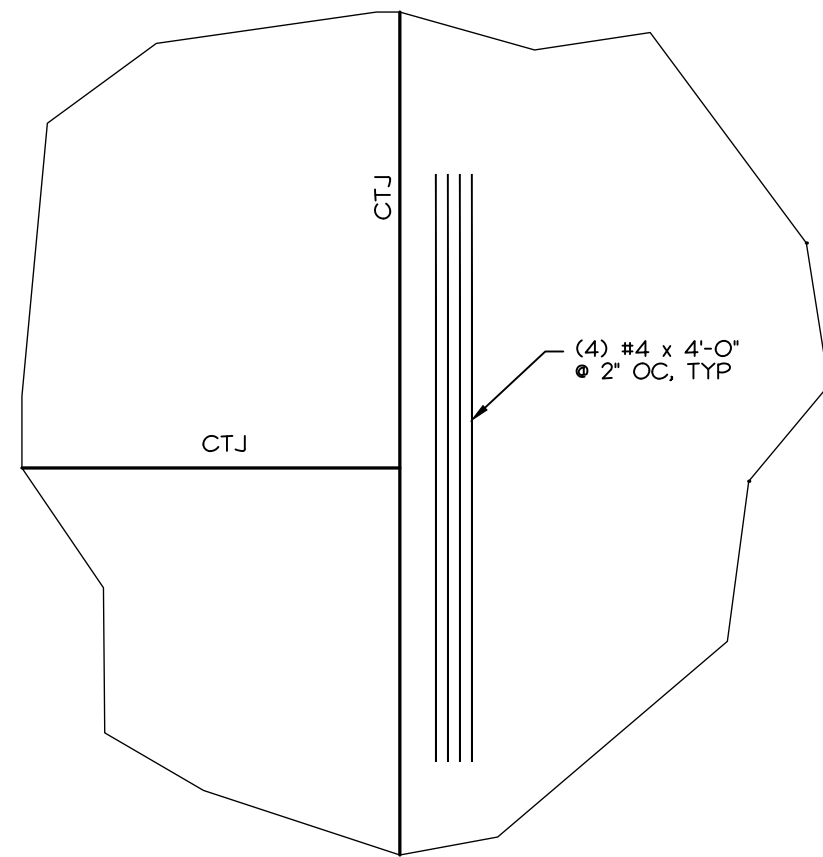
1 TYPICAL SLAB CONSTRUCTION
S3.1 3/4" = 1'-0"



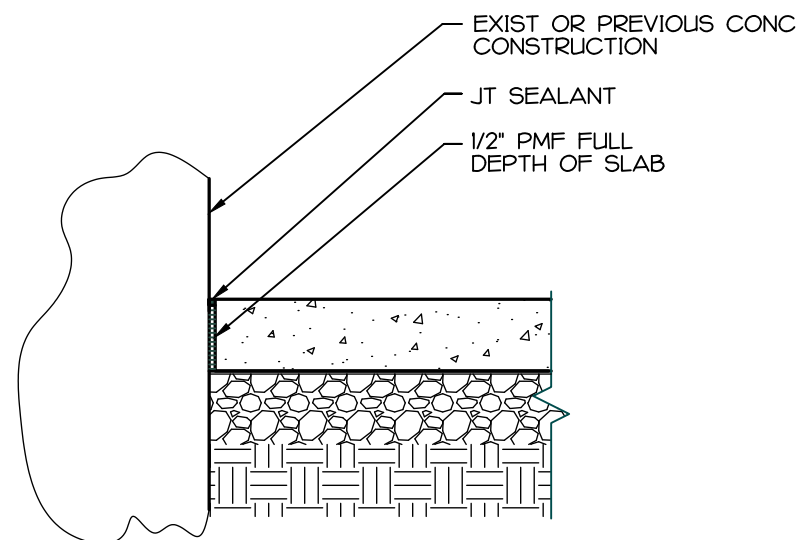
2 TYP. COLUMN ISOLATION JOINT (IJ)
S3.1 3/4" = 1'-0"



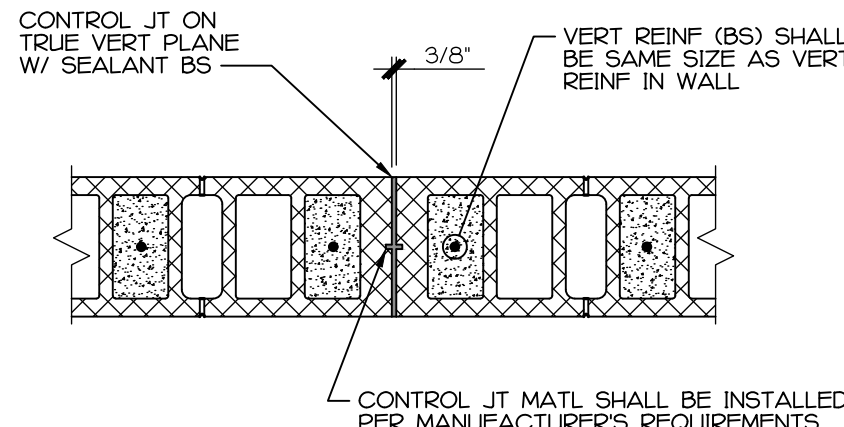
3 TYPICAL REINF. AT SLAB RE-ENTRANT CORNER
S3.1 3/4" = 1'-0"



4 CONTRACTION JOINT (CTJ) AT DISCONTINUOUS JOINT
S3.1 3/4" = 1'-0"

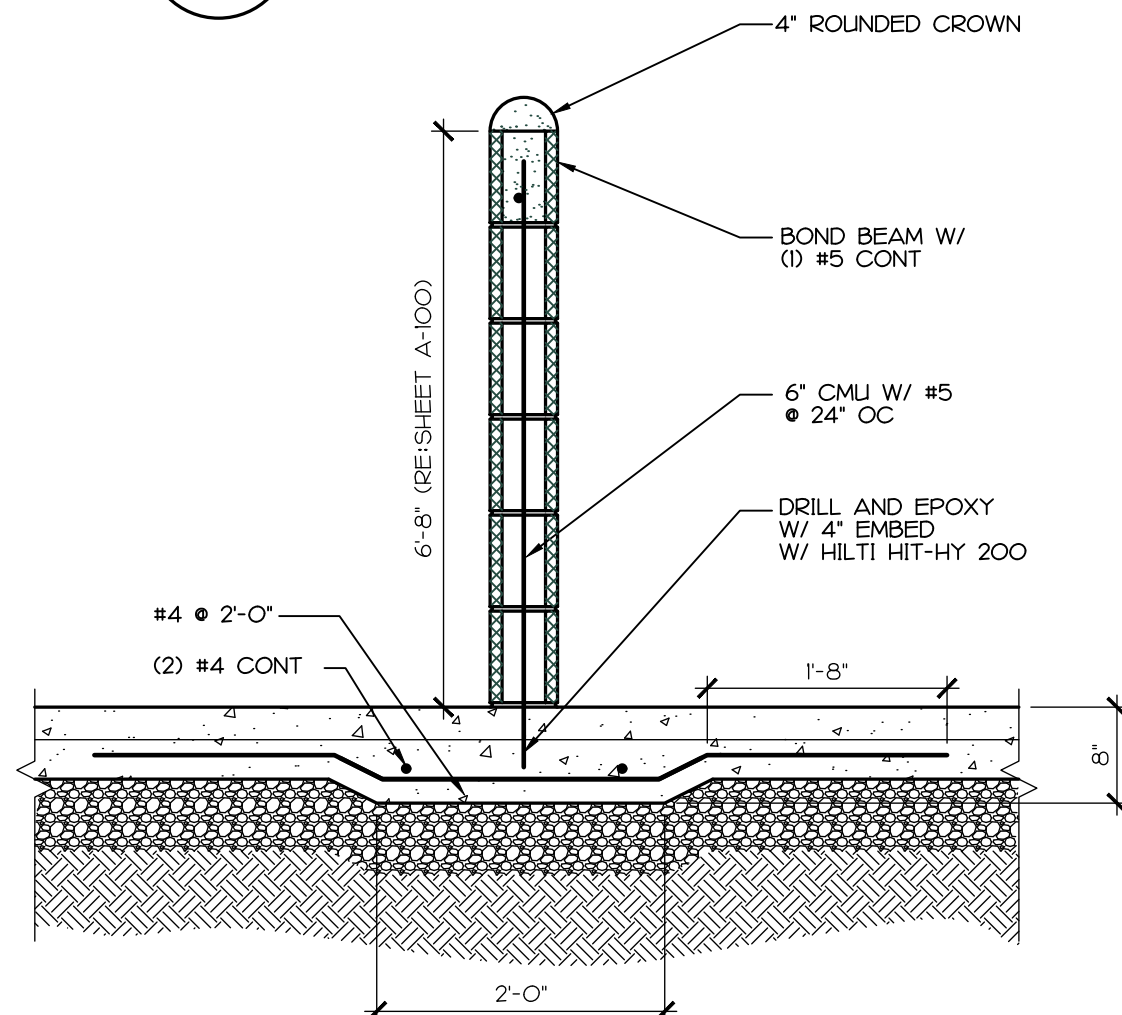


5 TYPICAL ISOLATION JOINT (IJ)
S3.1 3/4" = 1'-0"



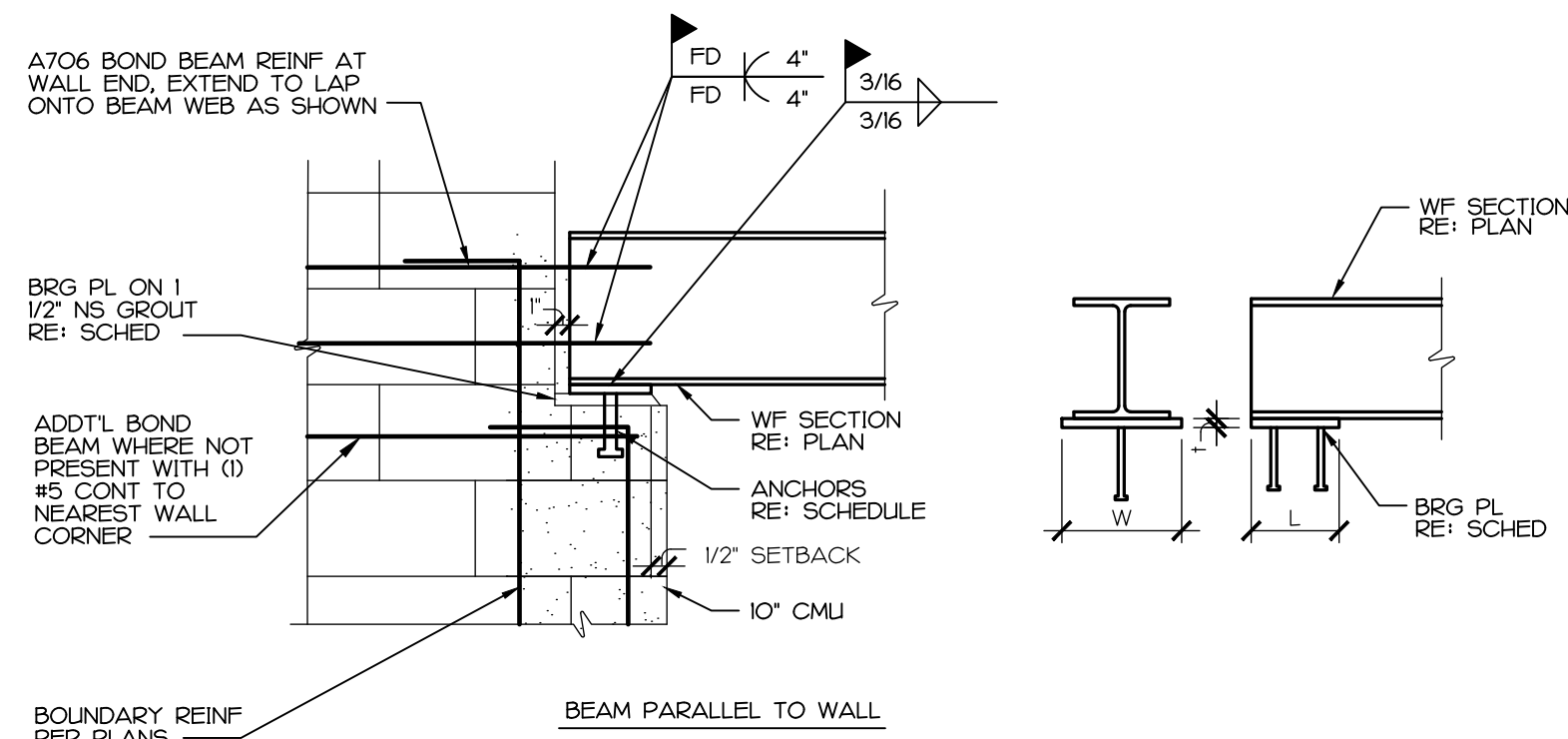
- NOTES:
1. CONTROL JOINT LOCATIONS SHALL BE SPACED APPROXIMATELY 25 FEET ON CENTER UNLESS NOTED OTHERWISE. REFER TO PLANS FOR CONTROL JOINT LOCATIONS.
 2. BOND BEAMS CONTINUOUS AT CLJ LINO

6 TYP. CMU CONTROL JT. DETAIL (CLJ)
S3.1 3/4" = 1'-0"



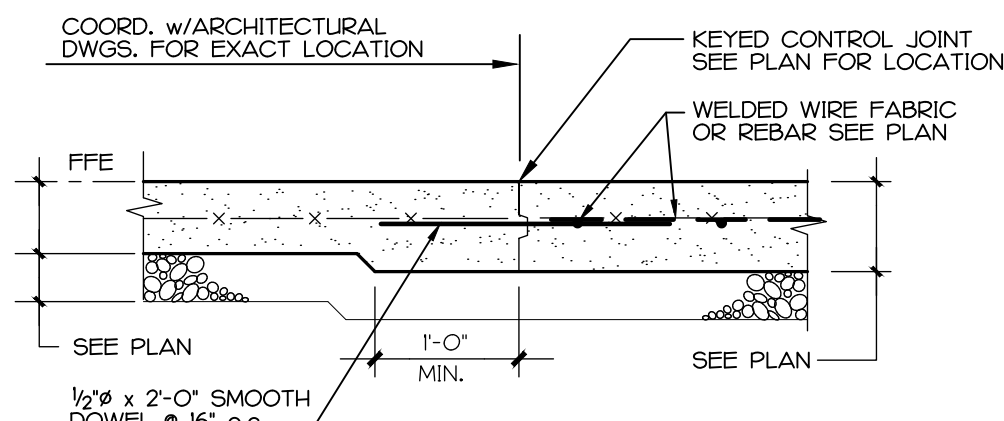
- NOTES:
1. INTERIOR CMU WALLS TO BE BUILT AFTER STORAGE RACKING SYSTEM IS INSTALLED. COORDINATE FINAL LOCATION OF WALLS WITH STORAGE RACKING LAYOUT.

7 TYPICAL THICKENED SLAB UNDER INTERIOR CMU WALL
S3.1 N.T.S.



BEARING PLATE SCHEDULE		
MARK	SIZE	ANCHORS
BRPI	1/2"x7 1/2"x0'-10"	(2) 1/2"DIA x 6' LG HEADED STUDS

8 TYP. BEAM SEAT IN CMU WALL
S3.1 N.T.S.



9 SECTION AT SLAB TRANSITION
S3.1 NO SCALE

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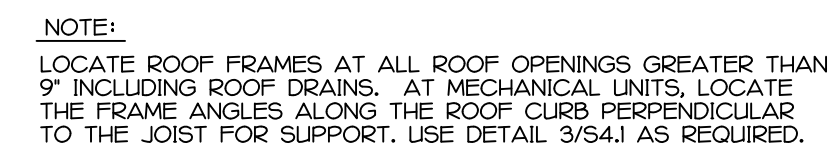
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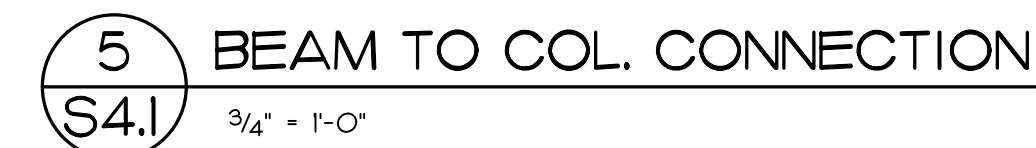
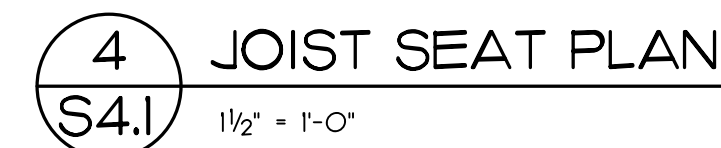
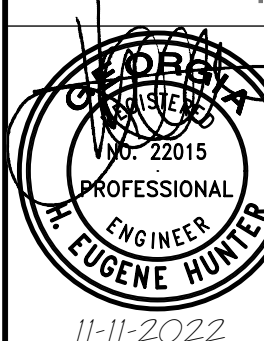
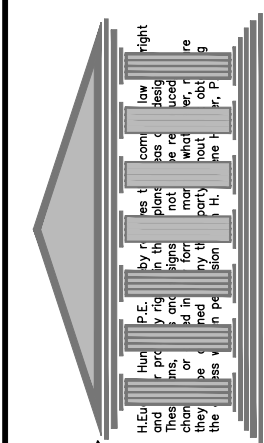
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S3.1





1 TYP. ROOF OPENING
S4.1 3/4" = 1'-0"

2 TYPICAL ROOF OPENING DETAIL
S4.I N.T.S.

[illegible]

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54.

BASED ON 8-BAY SINGLE SIDED PROTO. DATED APRIL 01 2022